

September 15, 2025**Addendum No. 05****File Reference Number: RFP 2025 069****Title: ONTC Bridge Maintenance – Temagami and Ramore Subdivision****RE: Clarifications/Questions**

Please refer to the following information/clarification:

QUESTIONS**Item 1:** Will the shims be supplied by ONTC?**Answer:** The size of shims is to be determined; pricing for the supply will be requested after size determination.**Item 2:** Will ONTC supply the spikes?**Answer:** ONTC will Supply spikes.**Item 3:** Please provide deck plans for the following subdivisions.**Answer:** Please refer to the following available drawings attached at the end of this Addendum.

<i>Temagami 115.50</i>	Basic
<i>Temagami 138.00</i>	None
<i>Ramore 7.45</i>	Basic except for Span 12 Detailed
<i>Ramore 15.10</i>	Basic
<i>Ramore 25.80</i>	Very Basic
<i>Ramore 58.30</i>	Very Basic
<i>Ramore 69.60</i>	None
<i>Ramore 79.00</i>	Detailed
<i>Ramore 96.00</i>	Basic
<i>Ramore 103.30</i>	Very Basic

Item 4: For shimming, would ONTC consider a Mark IV tamper solution?**Answer:** Yes, ONTC will consider a Mark IV tamper solution.

Item 5: For Ramore 58.30, can you please confirm that it is the north approach that needs to be undercut? From the pictures, it appears that the south approach has elevation issues

Answer: For 58.3 Ramore, it is only the North Approach. And, for 7.45, a review of an old survey indicates that the south approach may require undercutting too. Previous survey files of 7.45 and 58.3 Ramore are included along with the survey from 119.10 Temagami as separate attachments with this Addendum. Please find below the list:

- 58.3 RAMORE INITIAL COMPLETE (Addendum No. 05)
- 1050105 RSD 7.45 UTM 17 NAD83 CSRS-2D (Addendum No. 05)
- 1050105 RSD 7.45 UTM 17 NAD83 CSRS-3D (Addendum No. 05)
- 1050120-TEMAGAMI 119.10 UTM17 NAD83 CSRS 2D (Addendum No. 05)
- 1050120-TEMAGAMI 119.10 UTM17 NAD83 CSRS 3D (Addendum No. 05)

Item 6: Can ONTC please grant an extension for this RFP?

Answer: Yes. The revised submission deadline is now **Friday, September 19, 2025 at 2:00 p.m.**

Item 7: Please be advised that ONTC has updated Schedule A – Schedule of Quantities and Price – Part 3. This revised Schedule A for the Optional Items has been revised to now include additional undercutting at the South end of 7.45 Ramore Subdivision.

The RFP Documents have been revised and sections noted are affected below. The revised RFP sections supersede all previous RFP Documents versions for the said documents.

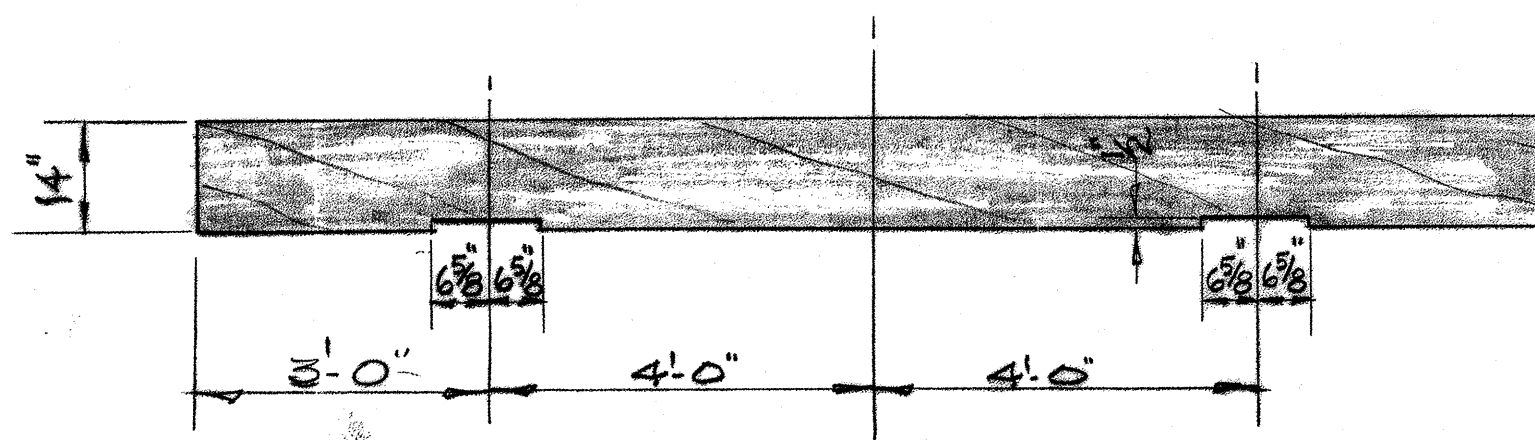
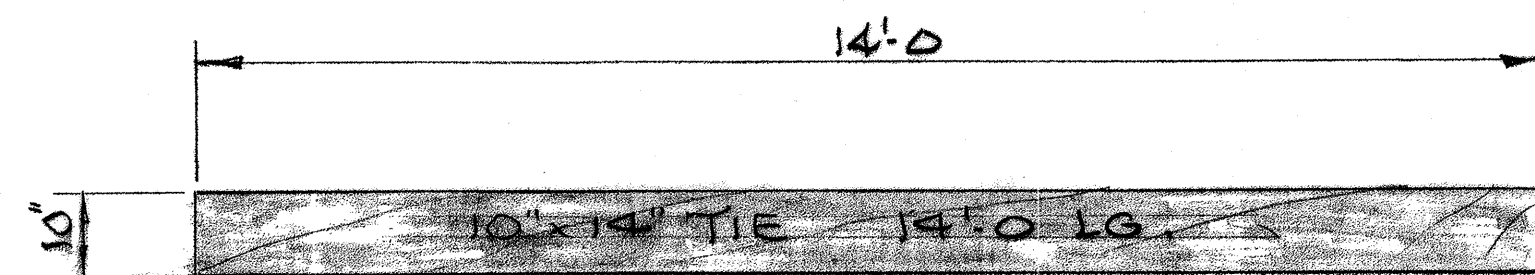
Delete Document:	Replace with Revised Document:
Schedule A – Schedule of Quantities and Prices – Part 3 – Optional Works	Schedule A – Schedule of Quantities and Prices – Part 3 – Optional Works – Rev.1

This Addendum hereby forms part of the RFP.

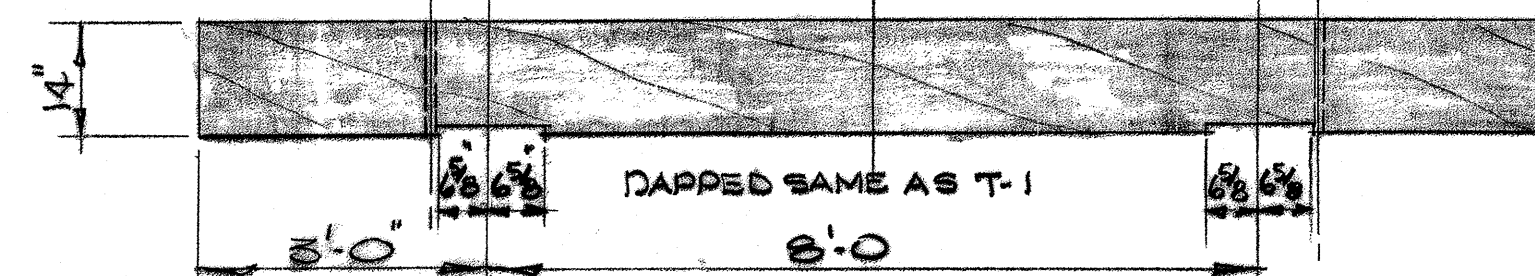
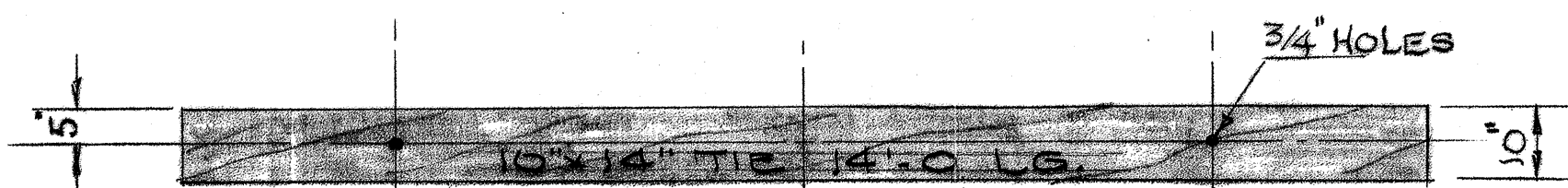
Regards,

Brinda Ranpura
Procurement Contracts Specialist
brinda.ranpura@ontarionorthland.ca

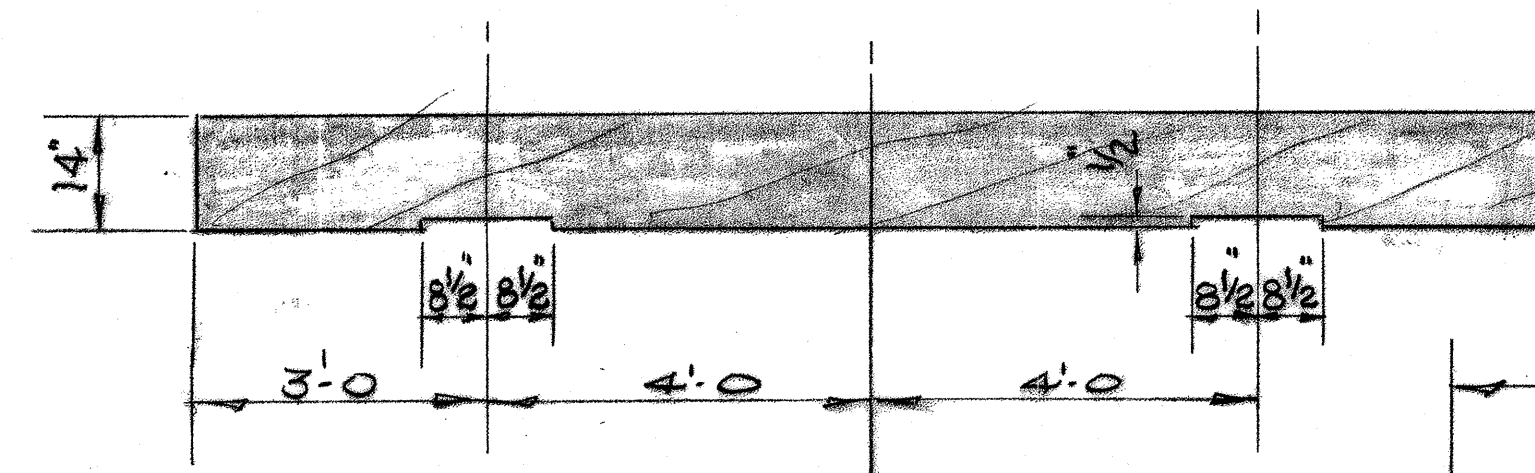
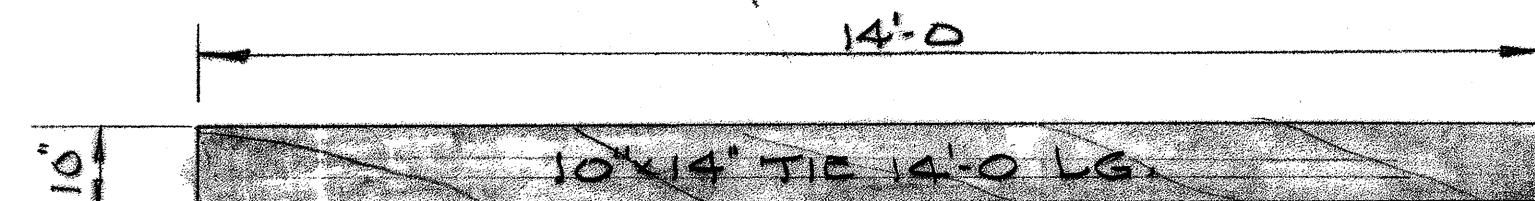
B-1308



T-1 114 REQ'D.



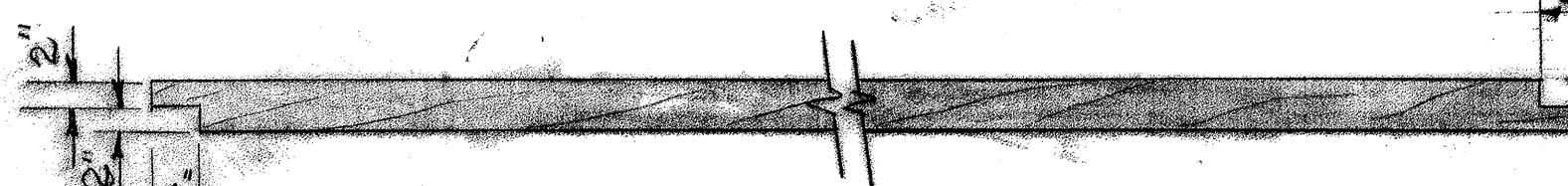
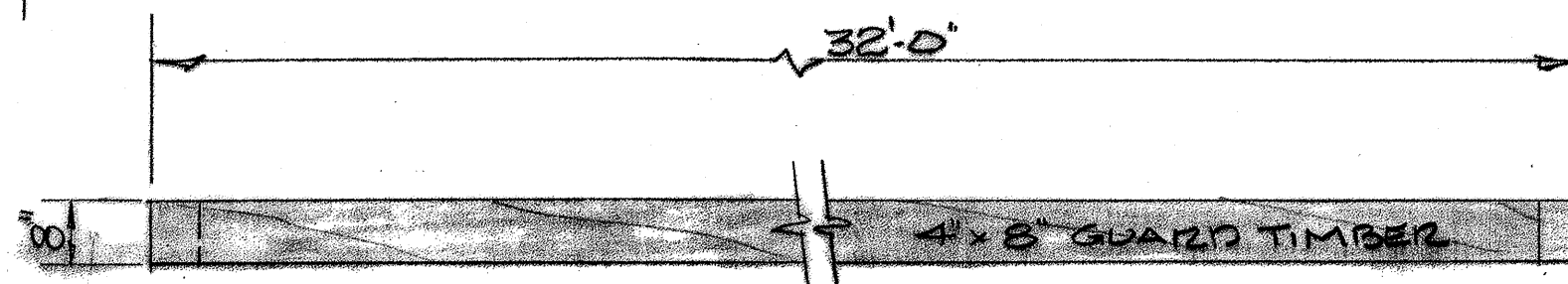
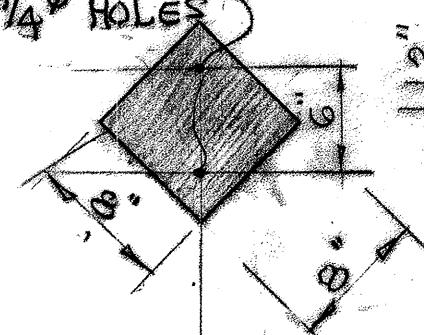
T-2 40 REQ'D.



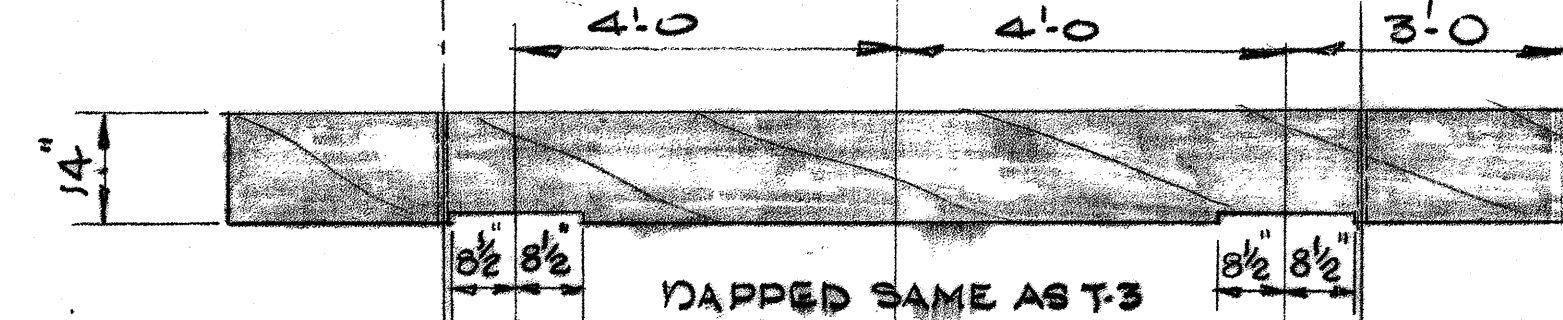
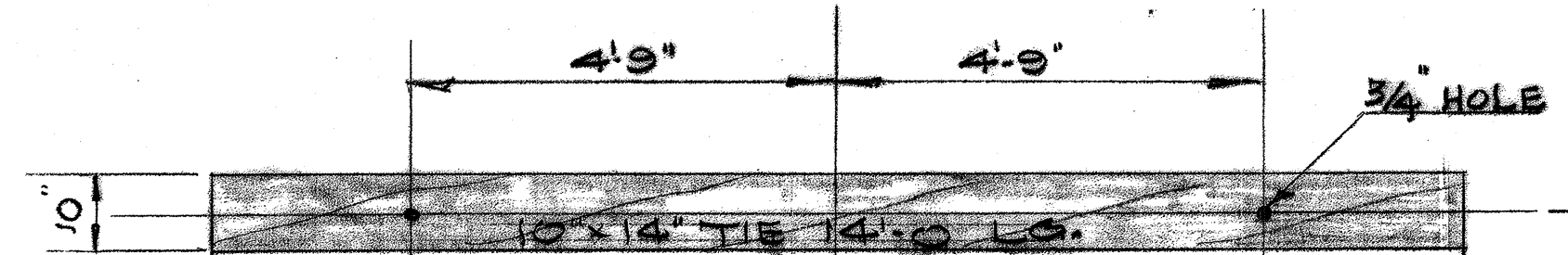
T-3 57 REQ'D.

SPACER BLOCKS
4"x8"-8"lg. fir
pressure treated
475 REQ'D.

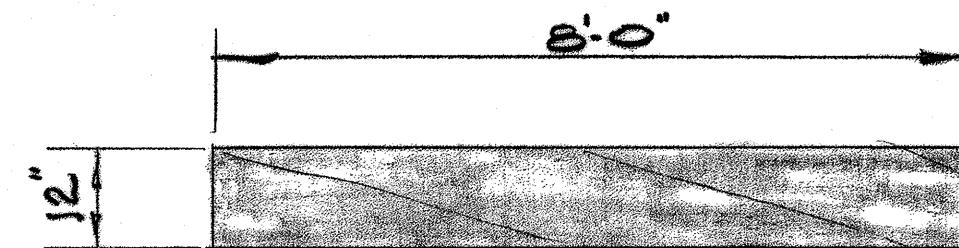
DRILL 1/4" HOLES



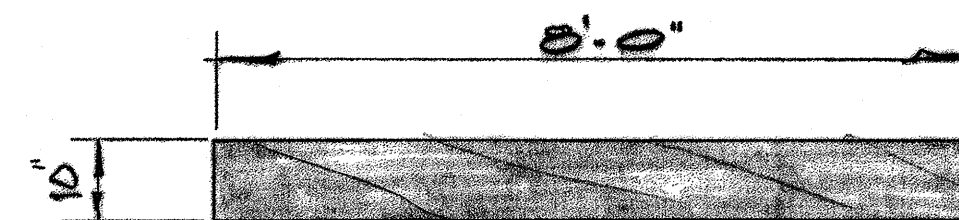
4x8 GUARD TIMBER 18 REQ'D.



T-4 20 REQ'D.



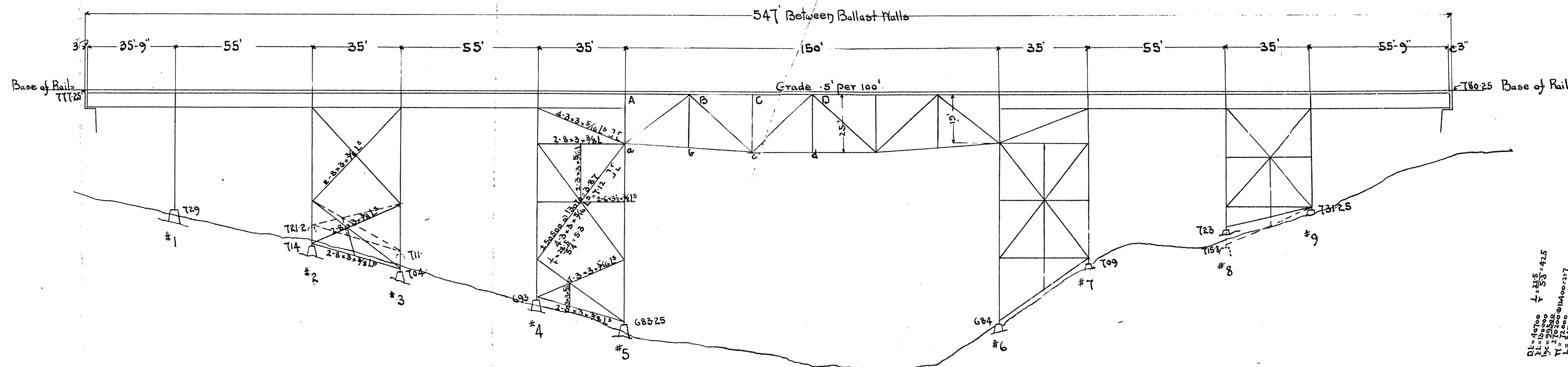
T-5 1 REQ'D.



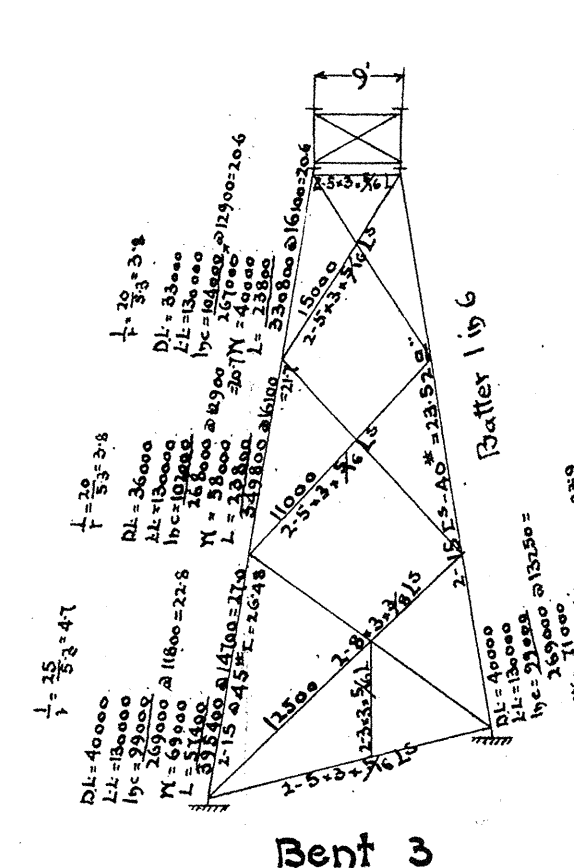
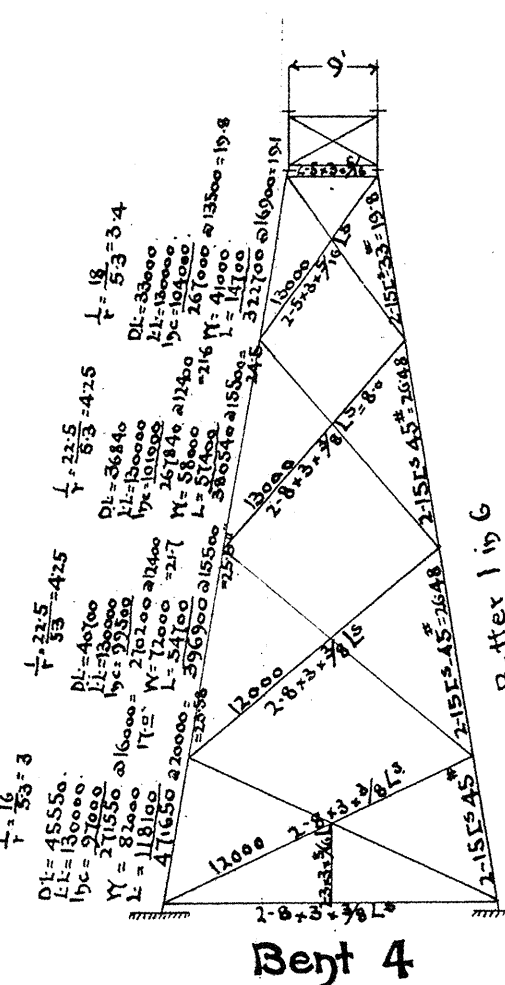
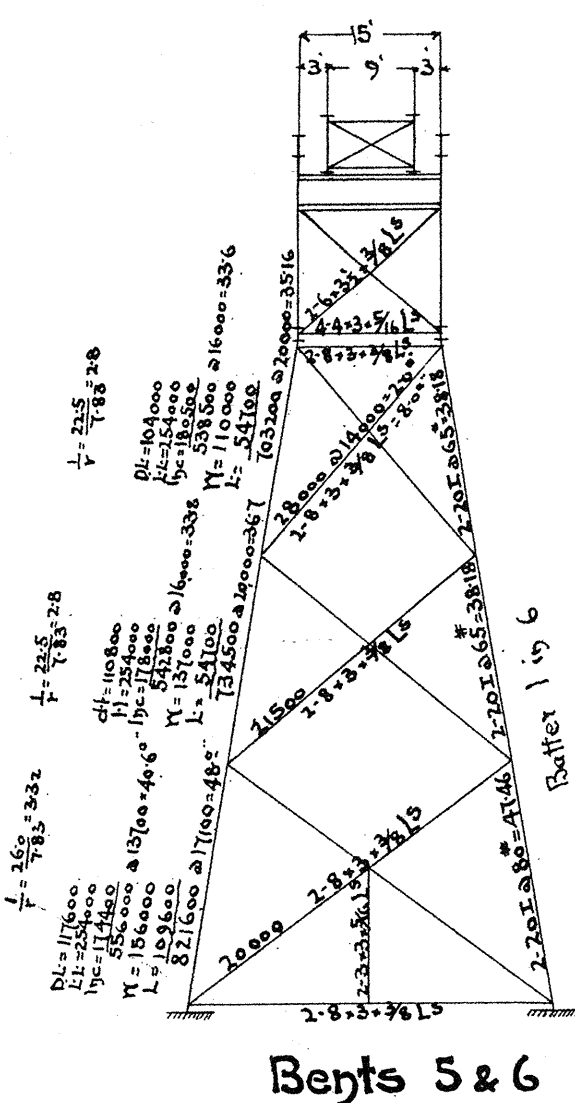
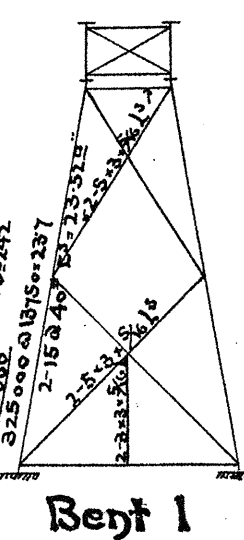
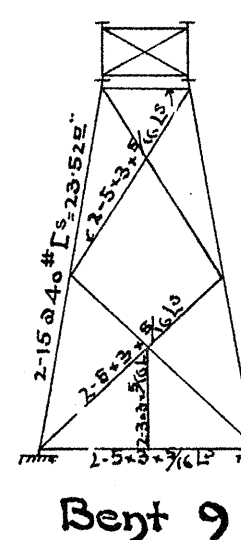
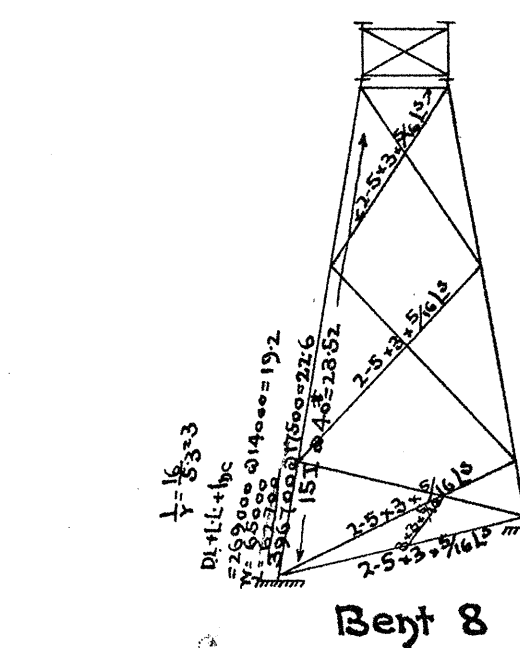
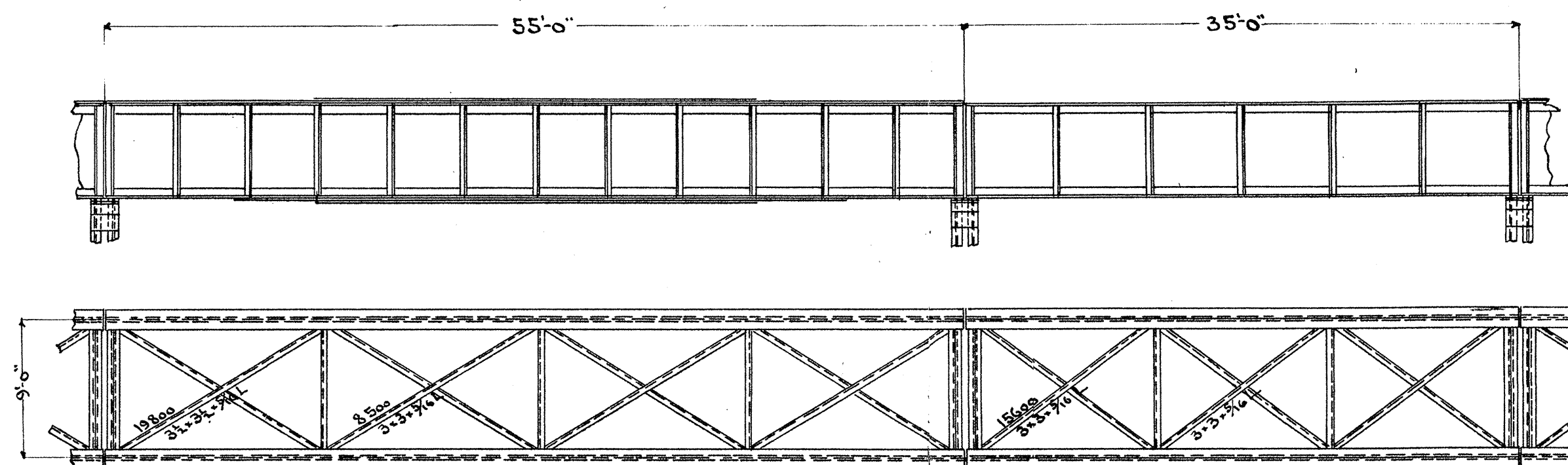
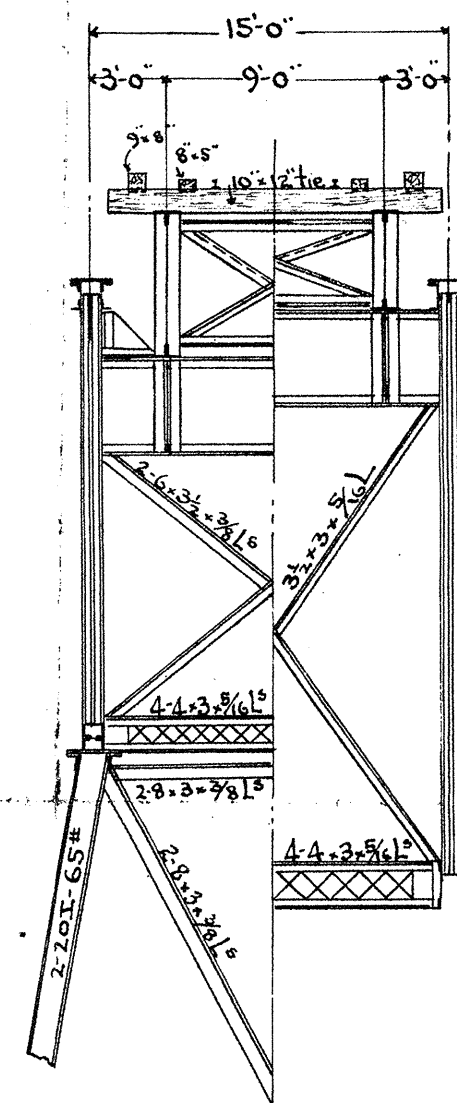
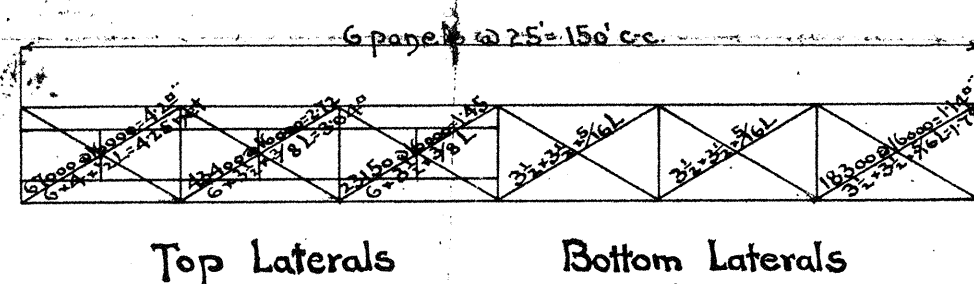
T-6 1 REQ'D.

NOTE:
ALL DRILLING DAPPING AND
CUTTING SHALL BE COMPLETED
PRIOR TO PRESSURE TREATMENT.

ONTARIO NORTHLAND R.L.WY.		
ENGINEERING DEPT. NORTH BAY ONT.		
DECK TIMBER DETAILS		
SOUTH WABSI BRIDGE MP 115.47		
TEMAGAMI SUBDIVISION		
drn	chk	
WDA		
approved	Struct. Supt.	
scale	date	no
as noted	June 7/78	B-1308



Member	Dead Load	Live Load	Total	Increment	Total	Unit Stress	Area	Material Used	Area	I
AB								4-3 1/2 x 3-3/8 L ^s = 9.2	11.45	
BCD	120000	272000	392000	189000	581000	13860	420	2-15 x 50 C ^s = 29.42	41.8	25
abc	84000	202000	286000	142000	428000	16000	26.8	1-22 x 3/4 plate = 12.38		57
cd	135000	311000	446000	217000	663000		41.6	4-6 x 4 x 3/4 L ^s = 21.76	26.76	net
aB	107000	251000	358000	176000	534000	14600	36.6	2-24 x 3/4 plates = 20.0	42.88	
Bc	50000	141000	191000	104000	295000	16000	18.4	4-6 x 4 x 3/4 L ^s = 16.72	37.72	31.9
cD	+21500	-47000	120100	145600	265700	11900	22.2	1-12 x 3/4 plate = 4.37	18.37	8.5
Aa	27000	97000	124000	76000	200000	11000	18.18	4-6 x 3 1/2 x 3/4 L ^s = 14.00	35.35	54.3
Bb, Dd								2-15 x 40 C ^s	23.52	
Cc	20000	85700	105700	69700	175400	10140	17.3	4-6 x 3 1/2 x 3/4 L ^s = 18	18.18	19
								12 x 3/4 pl. (Area not counted)	9.2	25
									18.0	3



End Elevation Cross Section

Floorbeam
 Moment $dI = 38450$
 $H = 256780$
 $I = 295430$
 $I_{pc} = 233000$
 $518430 \div 2.75 = 188000 \div 16000 = 11.75$
 $\% \text{ of Web} = 2.25$
 $2.6 \times 4 \times 3/4 L^s = 6.41$
 $8.72 \div 2 = 4.36$
 $48 \times 3/4 \text{ Web} = 18.0$

Stringer
 Moment $dI = 35200$
 $H = 250100$
 $I = 250950$
 $I_{pc} = 212000$
 $104950 \div 3.83 = 184000 \div 16000 = 11.5$
 $\% \text{ of Web} = 2.25$
 $2.6 \times 4 \times 3/4 L^s = 6.41$
 $11.74 \div 2 = 5.87$
 $48 \times 3/4 \text{ Web} = 18.0$

Shear
 $dI = 5620$
 $H = 65400$
 $I = 71020$
 $I_{pc} = 60100$
 $131120 \div 10000 = 13.11$
 $48 \times 3/4 \text{ Web} = 18.0$

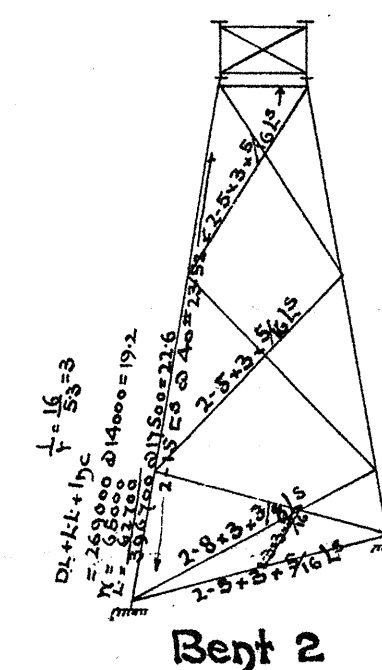
55' Span
 Moment $dI = 238000$
 $H = 1255000$
 $I = 1390000$
 $I_{pc} = 1080000$
 $2552000 \div 6 = 425500 \div 16000 = 26.6$
 $\% \text{ of Web} = 3.375$
 $2.6 \times 4 \times 3/4 L^s = 11.720$
 $2.15 \times 3/4 \text{ plate} = 12.38$
 $27.455 \div 2 = 13.7275$
 $72 \times 3/4 \text{ Web Pl.} = 27.0$

End Floorbeam
 Moment $dI = 65620$
 $H = 291000$
 $I = 356620$
 $I_{pc} = 238000$
 $594620 \div 4.16 = 142500 \div 16000 = 8.9$
 $\% \text{ of Web} = 2.54$
 $2.6 \times 4 \times 3/4 L^s = 6.41$
 $9.01 \div 2 = 4.505$
 $48 \times 3/4 \text{ Web} = 18.0$

Shear
 $dI = 21500$
 $H = 97000$
 $I = 118500$
 $I_{pc} = 79200$
 $197700 \div 10000 = 19.77$
 $54 \times 3/4 \text{ Web} = 20.3$

35' Span
 Moment $dI = 84000$
 $H = 523000$
 $I = 630000$
 $I_{pc} = 525000$
 $1208000 \div 5.86 = 206000 \div 16000 = 12.88$
 $\% \text{ of Web} = 3.375$
 $2.6 \times 4 \times 3/4 L^s = 11.720$
 $12.885 \div 2 = 6.4425$
 $72 \times 3/4 \text{ Web Pl.} = 27.0$

Shear
 $dI = 9620$
 $H = 100000$
 $I = 88620$
 $I_{pc} = 70400$
 $159020 \div 10000 = 15.9$
 $72 \times 3/4 \text{ Web Pl.} = 27.0$



Submitted by: D. J.
 Date: Sept 20, 08
 Project #:
 Drawn by:
 Checked by:
 File #:

Temiskaming & Northern Ont. Ry.
 Trestle
 North Crossing Blanche River
 Strain Sheet

Scales (1/4" = 1')
 Drawing "A"

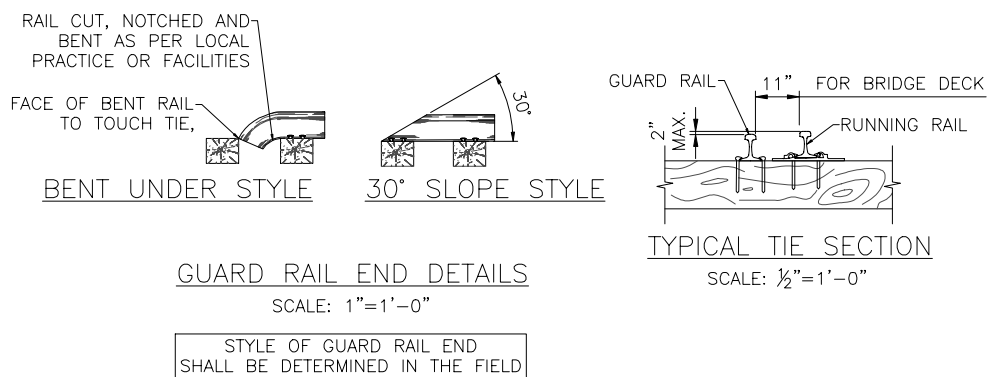
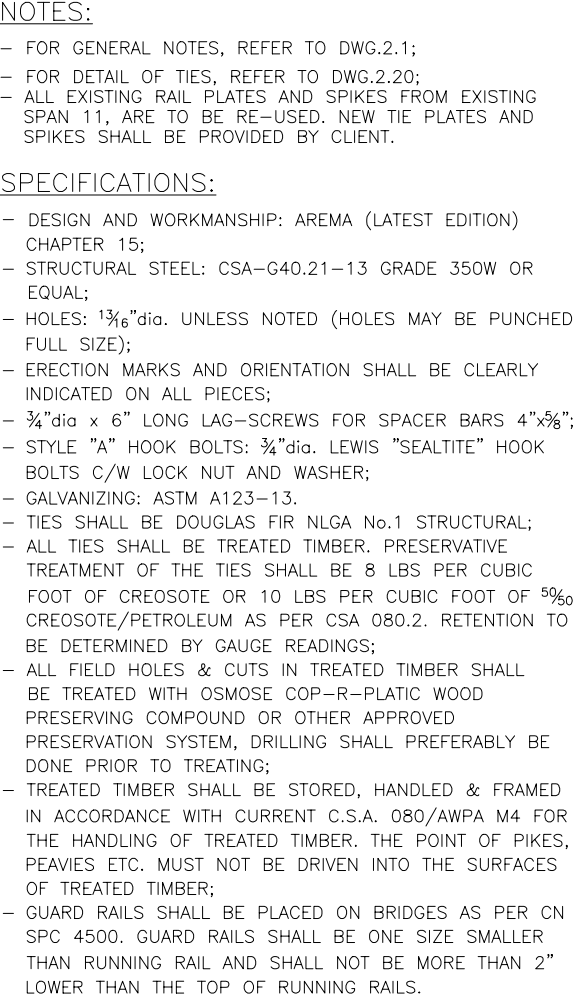
Dom Bridge Co Ltd
 Ladysmith P.Q.

E 46.8

J = 377 kN/m
 = 46.8 kN/m

1.9
 1.15 = 0.62 x 1.9
 = 1.15

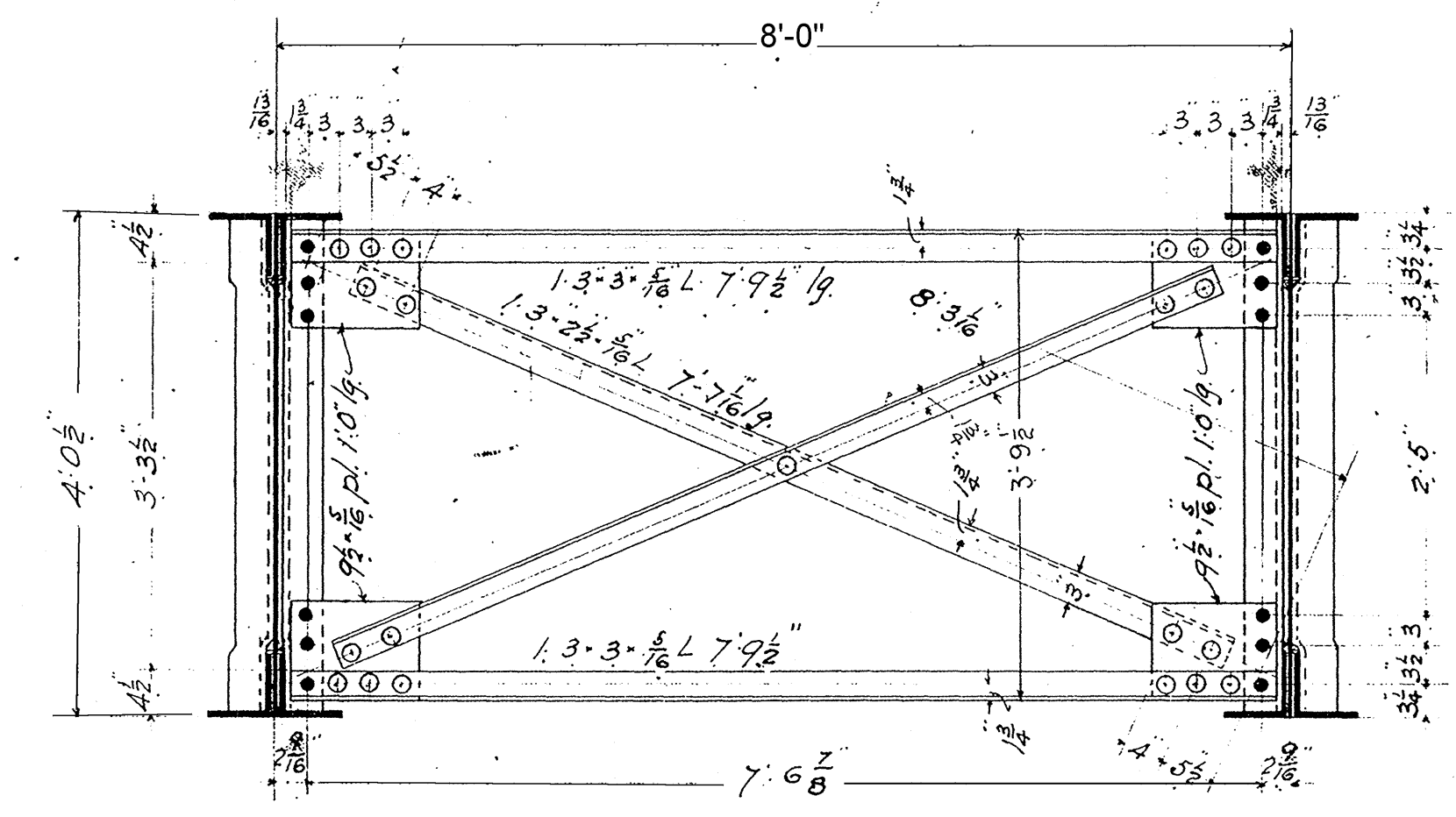
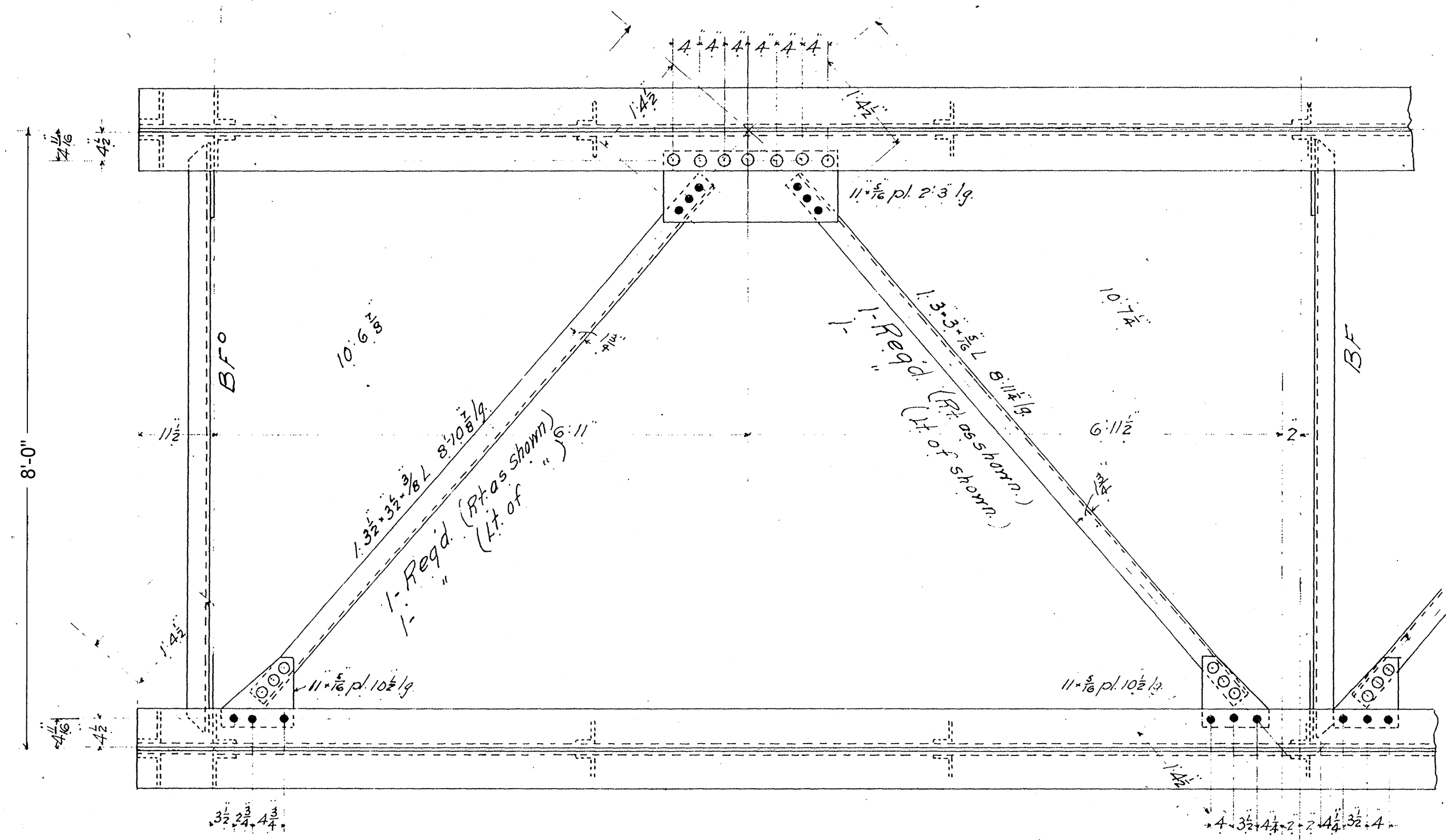
957



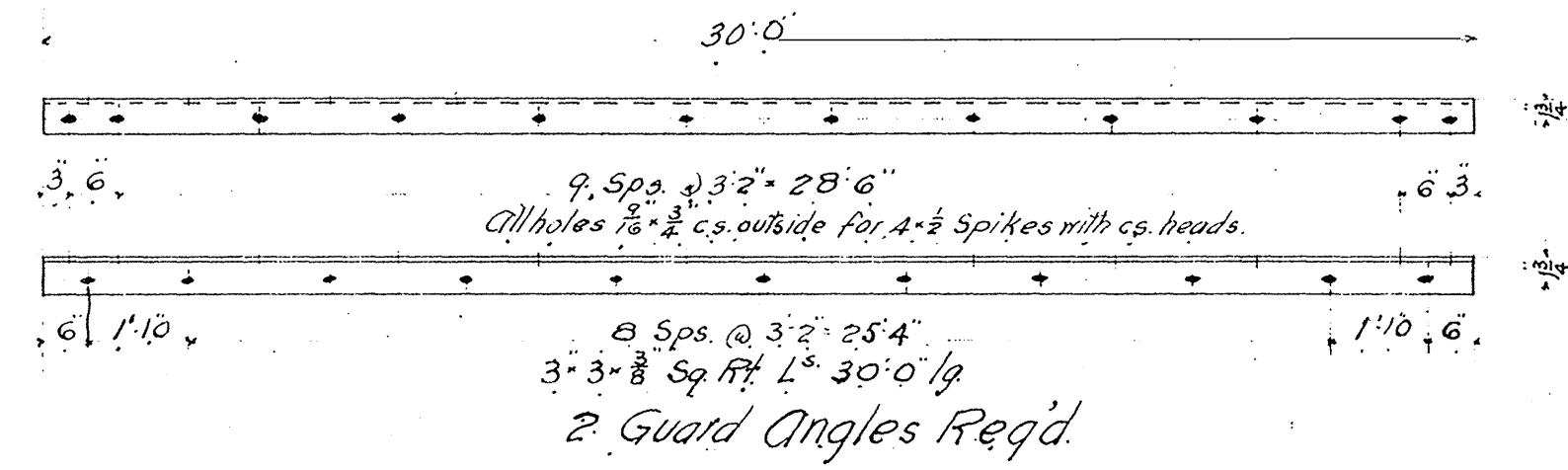
QUANTITIES:

- 3/4"dia.x6" LAG SCREW _____	116 EA.
- 3/4"dia.x8" LAG SCREW FOR SPLICE _____	12 EA.
- 3/4"dia.x1'-5" LONG HOOK BOLT _____ (C/W HARDWARE)	40 EA.
- GUARD RAIL _____	2-39 ft RAILS

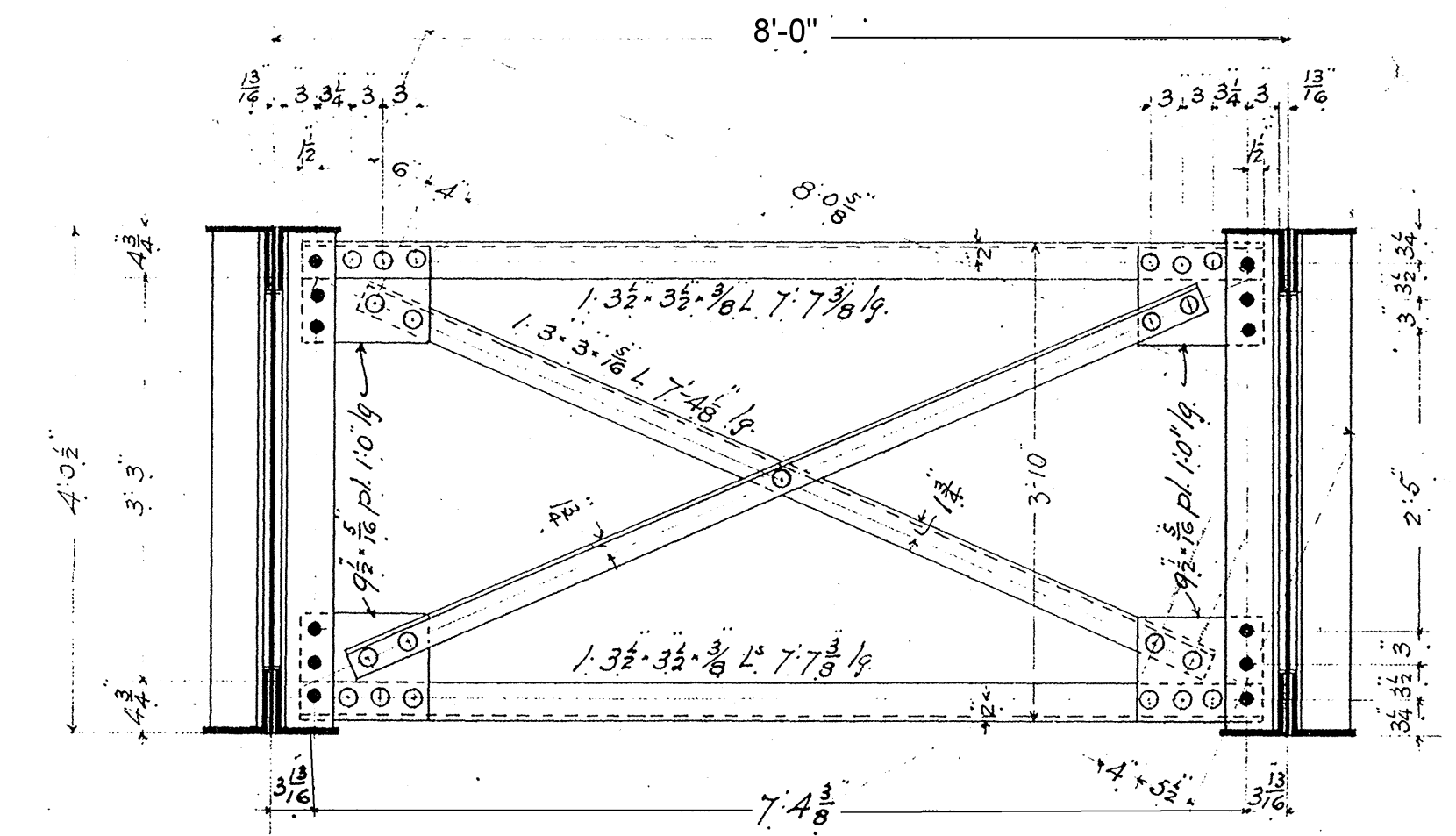
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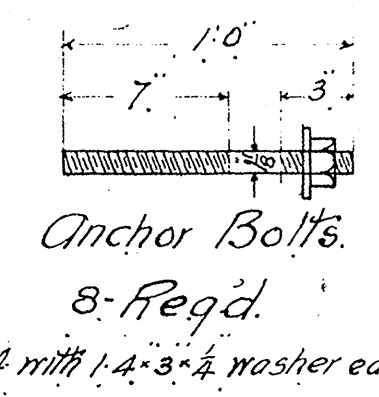
1. Int. Brace Frame Req'd
Mark "B.F."



2. Guard Angles Req'd



2. End Brace Frames Req'd
Mark "B.F."



- Floor Iron**
- 16 $\frac{3}{8}$ ϕ bolts 20' Grip 1'-9 3/4" lg
 - 16 $\frac{3}{8}$ ϕ coach screws 9" lg
 - 32 C.I. washers for $\frac{3}{8}$ ϕ bolts
 - 16 Nut washers for coach screws
 - 56 $\frac{1}{2}$ " Spikes 8" lg
 - 46 $\frac{3}{8}$ ϕ Spikes 4" lg csk heads

$\frac{7}{8}$ " Rivets. Holes in bottom flanges punched $\frac{13}{16}$ " and reamed to $\frac{15}{16}$ "

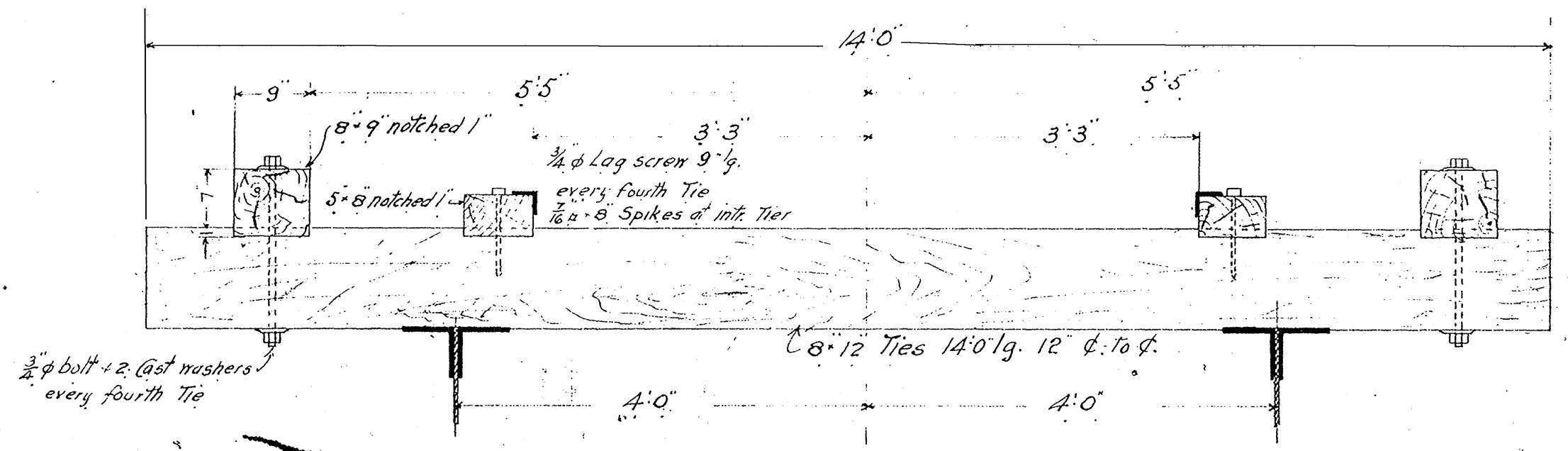
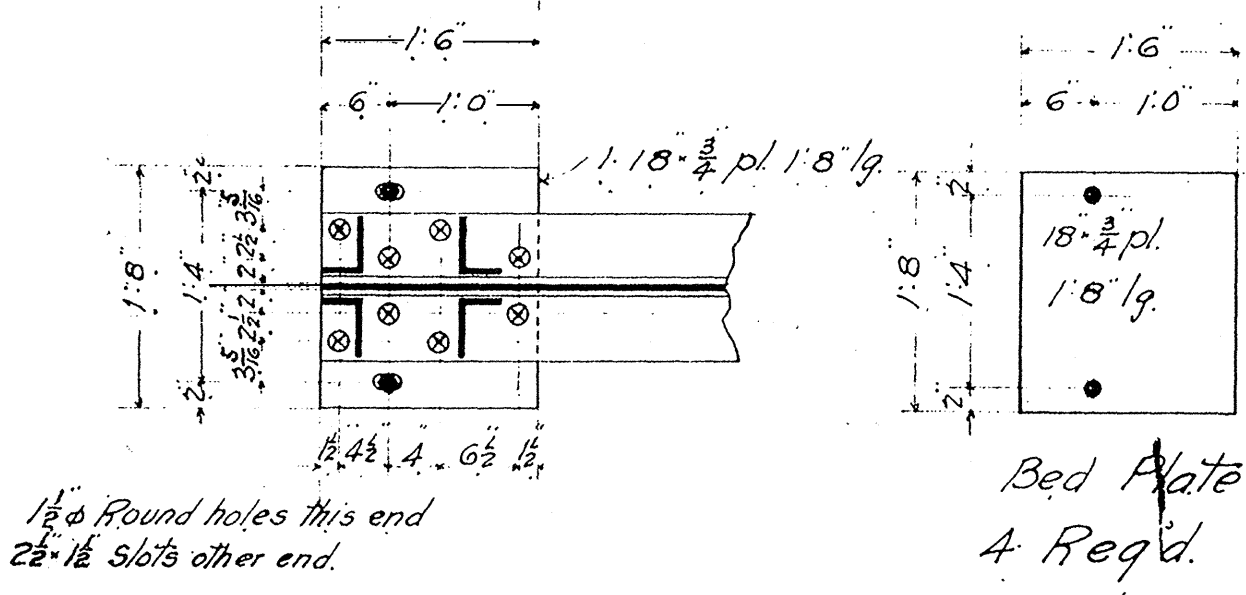
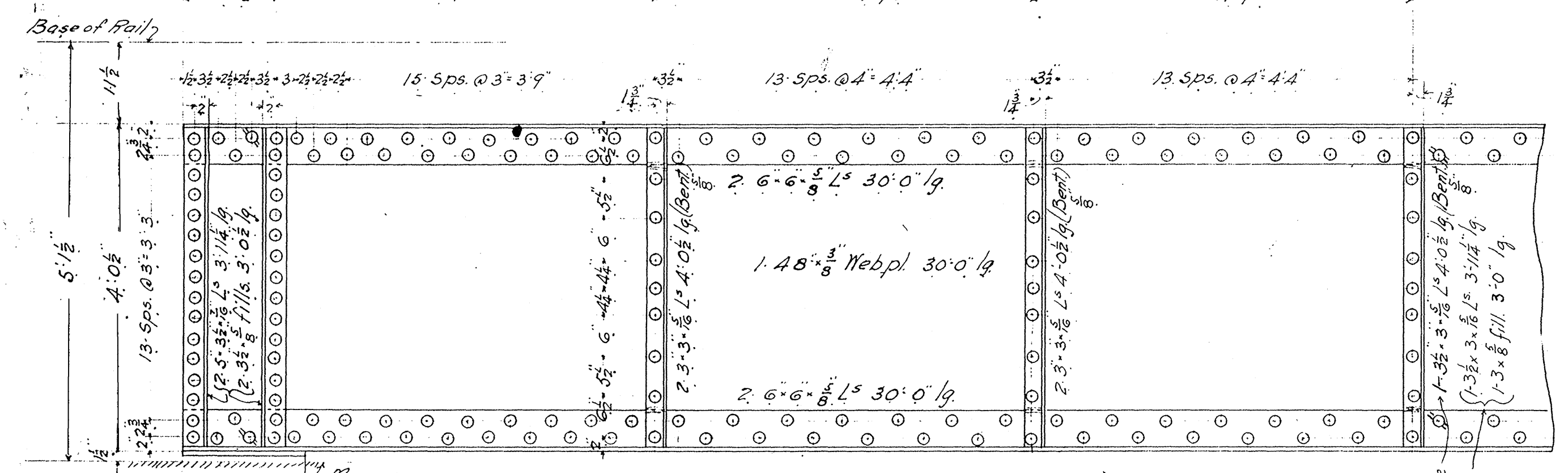
Span shop rivetted up complete

Contract 2560
Tamskaming & Northern Ontario
Railway Co.

30'-0" Plate Girder Span
at 31st Mile
Drawing No. 1

July 20, 04
AL

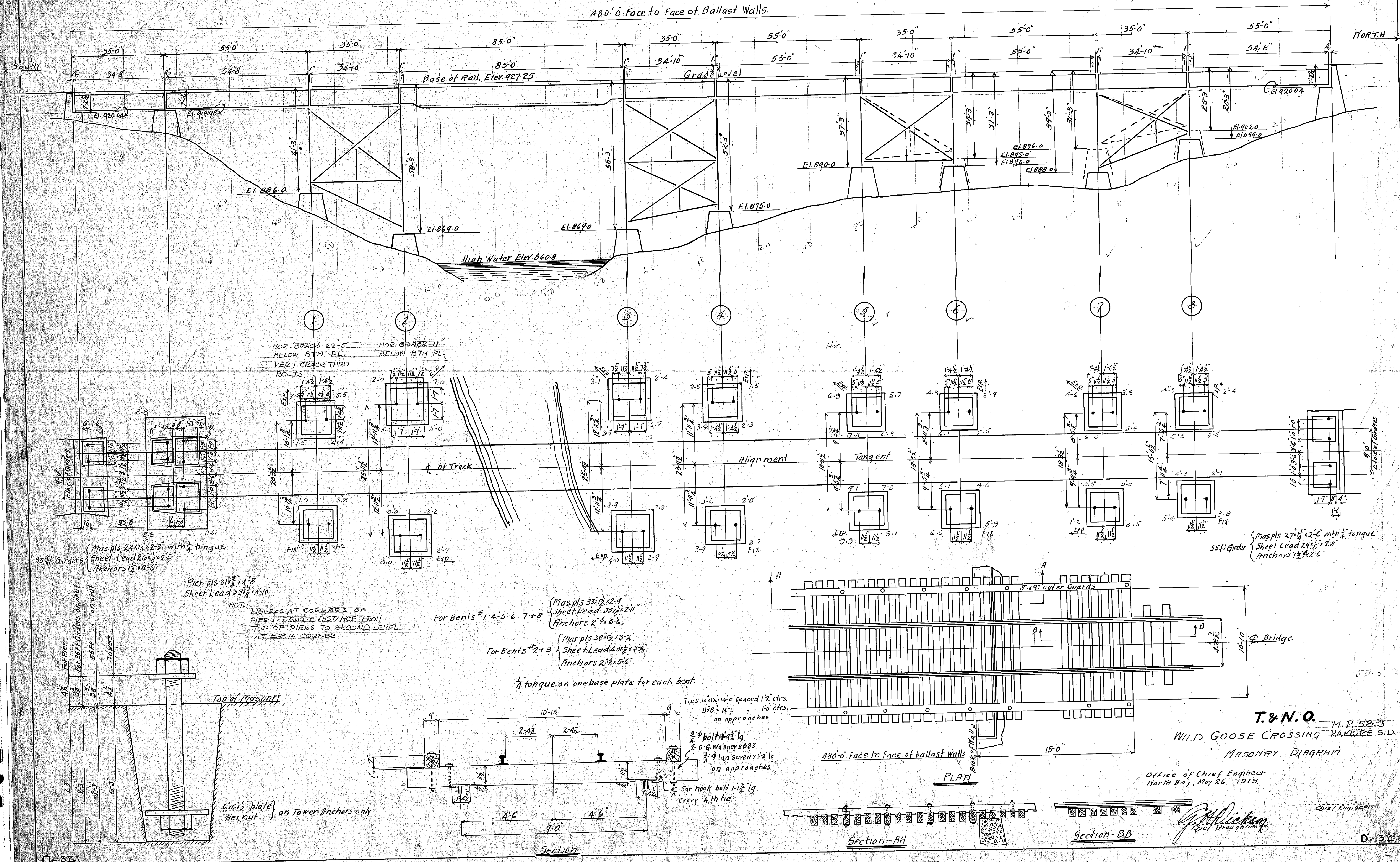
July 18/04 115.

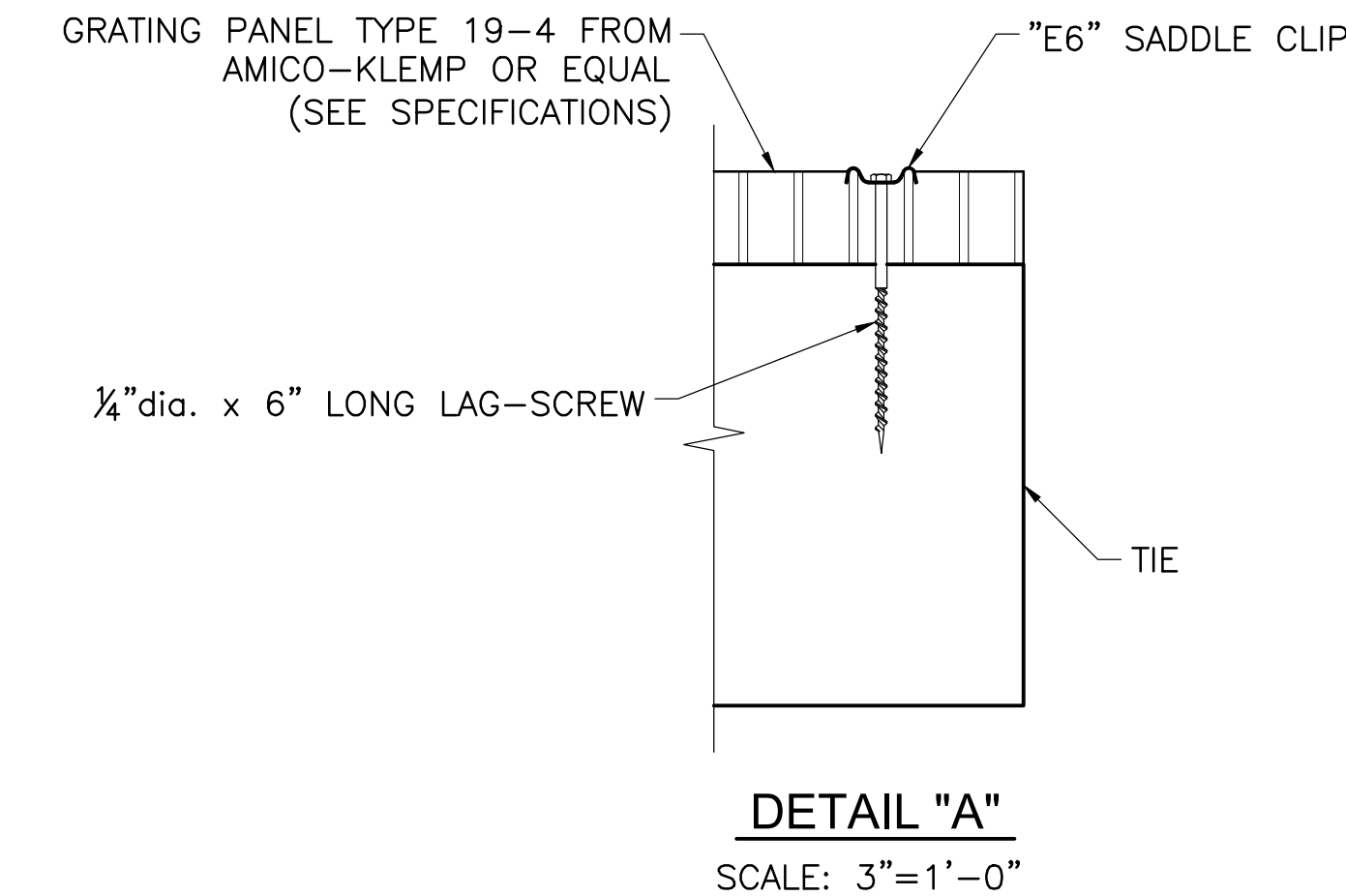


1 1/2" Round holes this end
2 1/2" Slots other end.

Bed Plate
4 Regd.

$\frac{3}{8}$ ϕ bolt + 2 flat washers
every fourth Tie



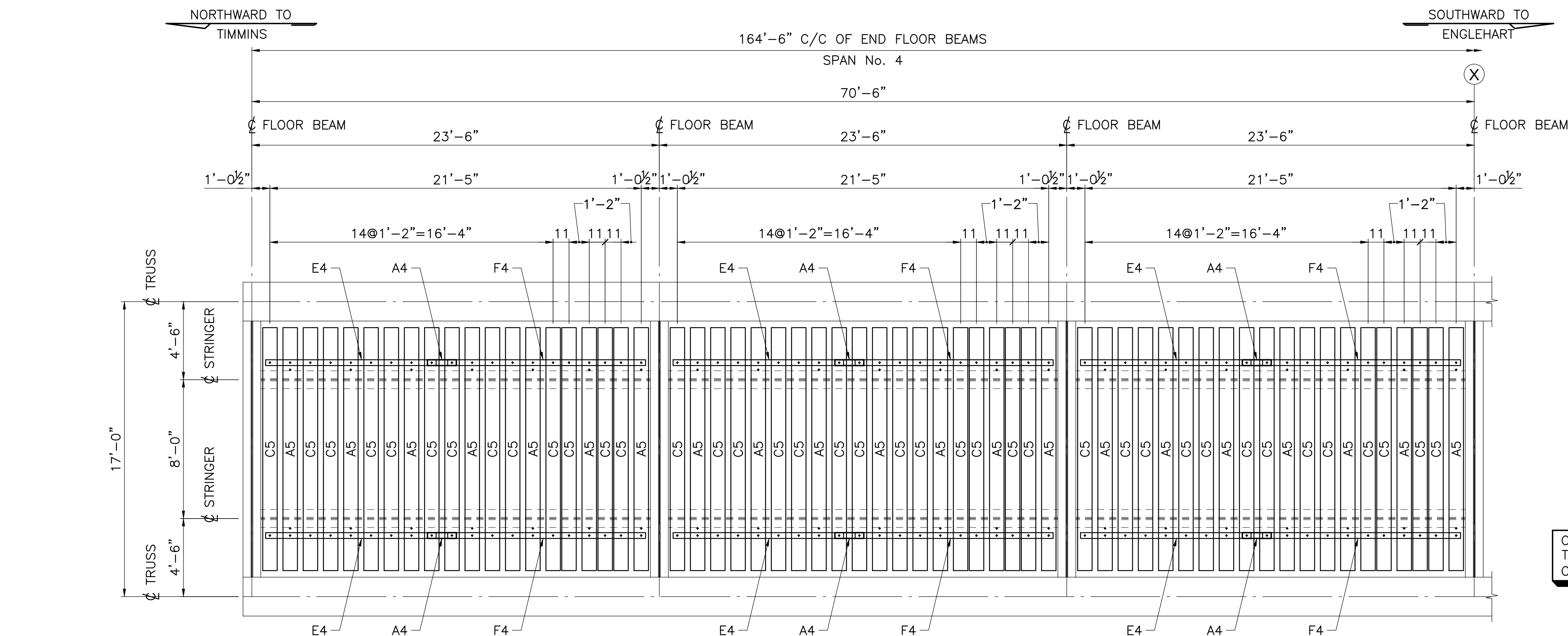


- | | |
|--|---------|
| -3/4"dia. x 6" LAG-SCREWS | 810 EA. |
| -3/4"dia. x 8" LAG-SCREWS | 80 EA. |
| -3/4"dia. x 1'-6" HOOK BOLT | 320 EA. |
| -1/4"dia. x 6" LAG-SCREWS FOR "E6" SADDLE CLIP | 65 EA. |

ONCE HOOK BOLTS ARE INSTALLED,
THREADED END OF BOLT SHOULD BE
CUT OFF APPROXIMATELY $\frac{1}{4}$ " ABOVE NUT

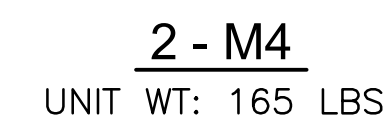
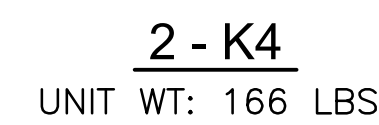
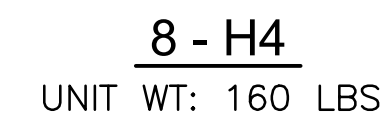
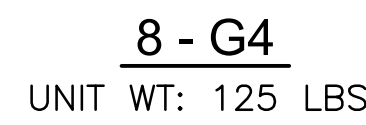
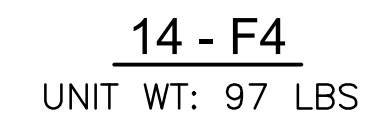
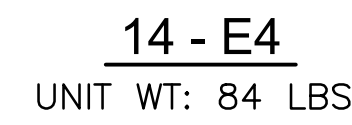
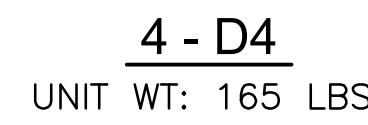
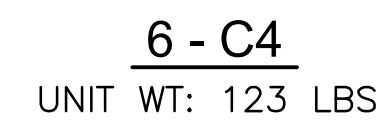
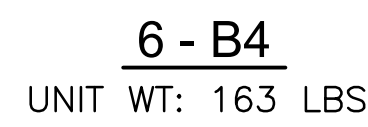
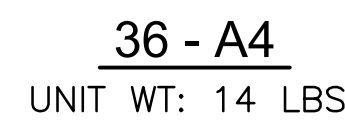
TYPICAL CROSS SECTION AT REFUGE BAY

DWG. BY: ED	CHK BY: WW	OFFICE FILE:
		SCALE: $\frac{1}{4}"=1'-0"$
ENGINEER:		DATE: MAY 2016
		PLAN No.
PROJECT MANAGER:		<i>RMR-79.00-2.1</i>



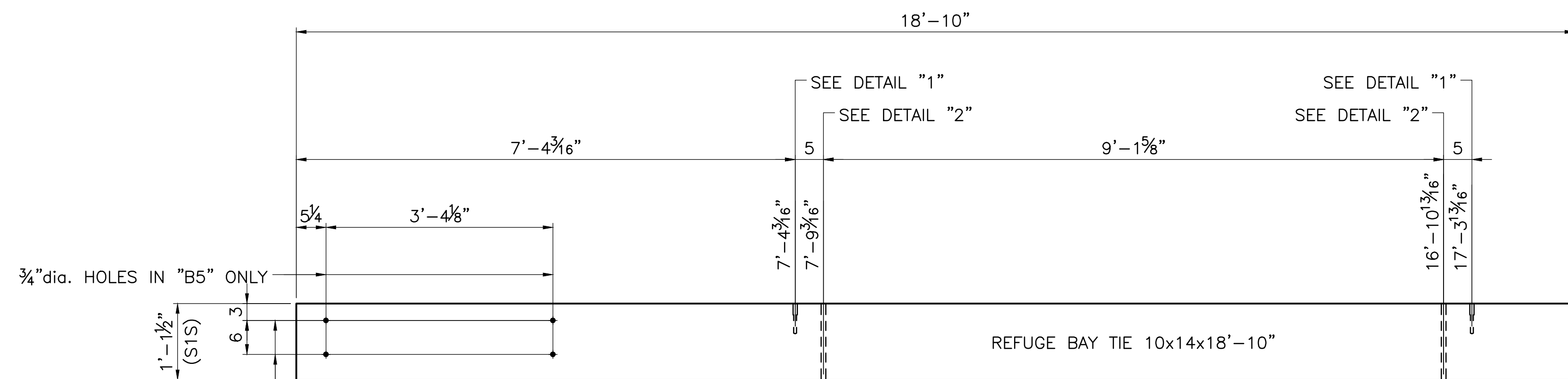
ERECTION DIAGRAM

BRIDGE OVER DRIFTWOOD RIVER Mile 79.00 RAMORE SUBDIVISION DECK REPLACEMENT ERECTION DIAGRAM SPAN 4		
DWG. BY: ED	CHK BY: WW	OFFICE FILE:
ENGINEER:		SCALE: $\frac{1}{4"} = 1' - 0"$
		DATE: MAY 2016
PROJECT MANAGER:		PLAN No. <i>RMR-79.00-2.2</i>

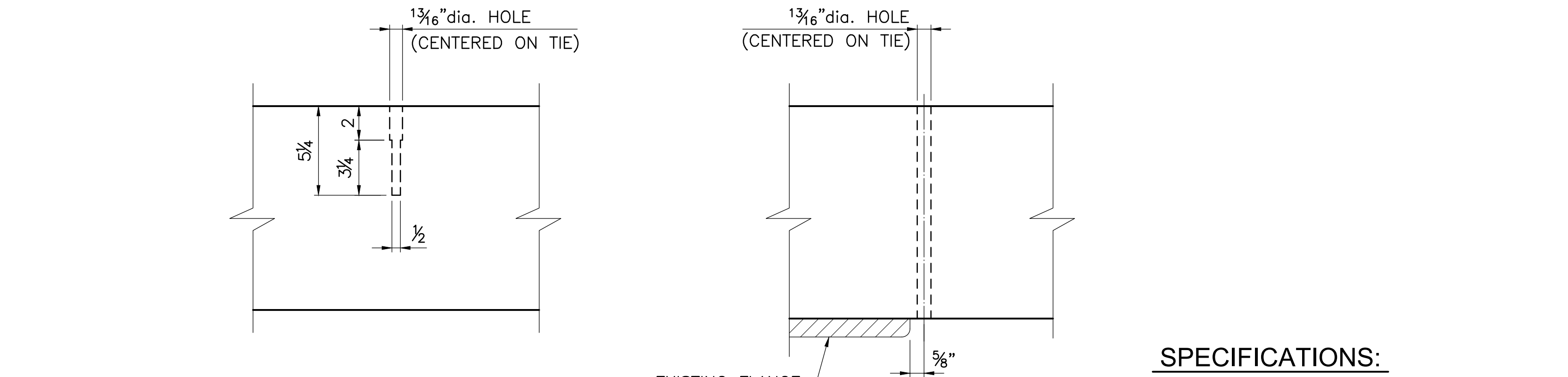


NOTES:
-FOR NOTES AND SPECIFICATIONS SEE DRAWING No.-2.1

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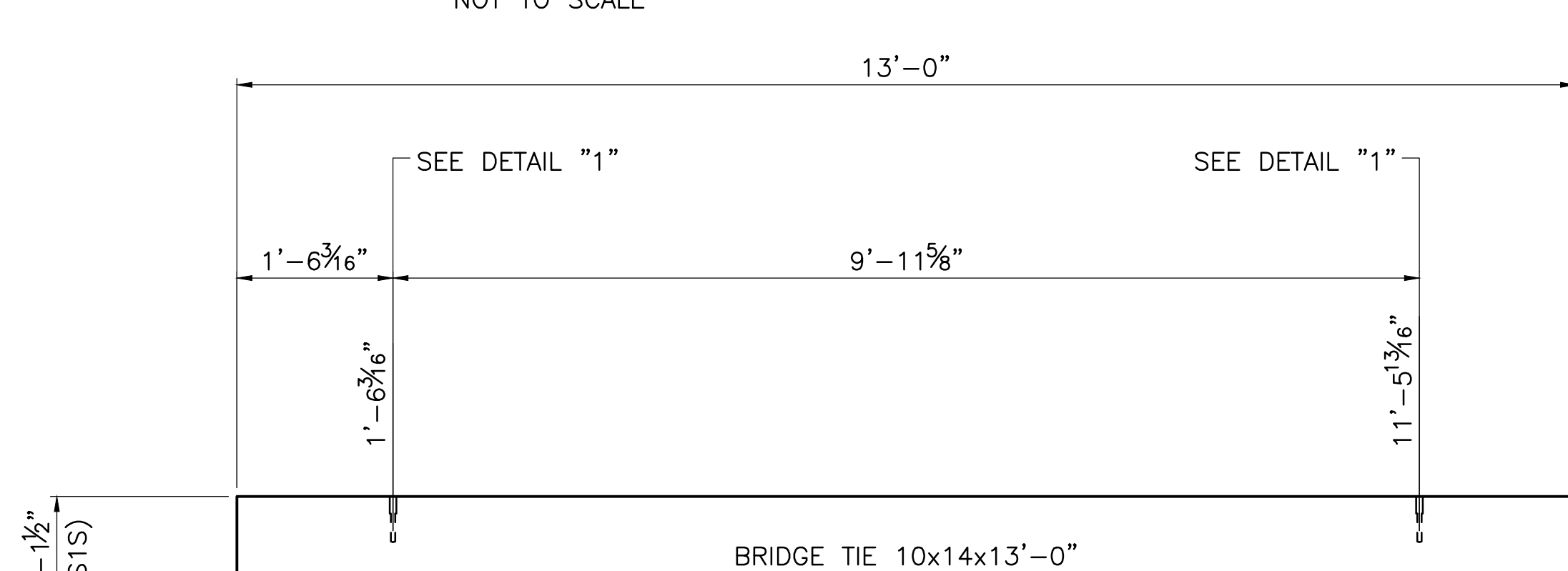


REFUGE BAY TIE "F5"
4—REQUIRED



DETAIL "2"

TYPICAL DETAIL FOR HOOK BOLT HOLES
NOT TO SCALE



BRIDGE TIE WITHOUT HOOK BOLTS "E5"
176-REQUIRED

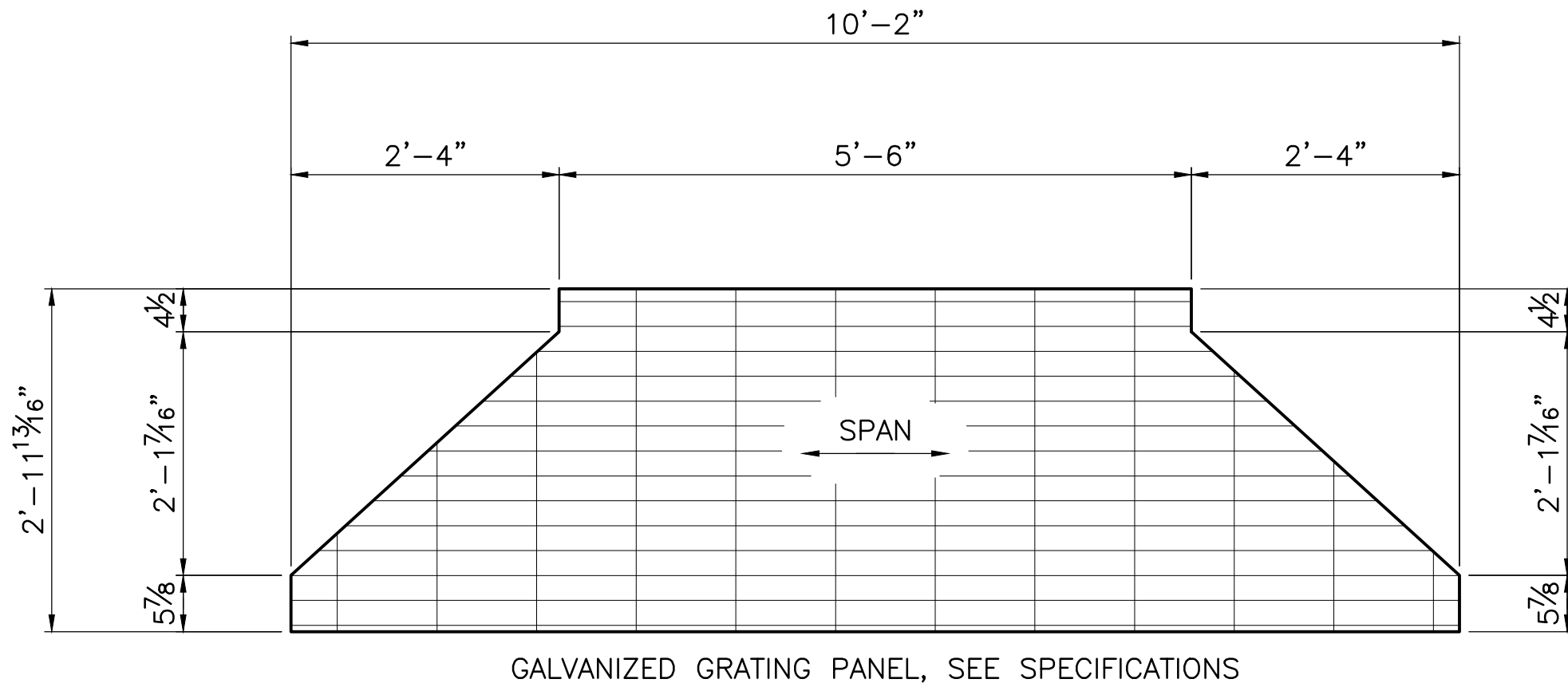


- TIES TO BE CUT AND SIZED AS SHOWN
- TIES SHALL BE TREATED DOUGLAS FIR CLASS L-57B NLGA 130B
- ALL TIES SHALL COME WITH ANTI-SPLIT PLATES INSTALLED AT BOTH ENDS
- ALL TIES SHALL BE TREATED TIMBER. PRESERVATIVE TREATMENT OF THE TIES SHALL BE 8 LBS PER CUBIC FOOT OF CREOSOTE OR 10 LBS PER CUBIC FOOT OF 50/50 CREOSOTE / PETROLEUM AS PER CSA 080.2. RETENTIONS TO BE DETERMINED BY GAUGE READINGS.
- ALL FIELD HOLES & CUTS IN TREATED TIMBER SHALL BE TREATED WITH OSMOSE COP-R-PLASTIC WOOD PRESERVING COMPOUND OR OTHER APPROVED PRESERVATION SYSTEM, DRILLING SHALL PREFERABLY BE DONE PRIOR TO TREATING.
- TREATED TIMBER SHALL BE STORED, HANDLED & FRAMED IN ACCORDANCE WITH CURRENT C.S.A. 080/AWPA M4 FOR THE HANDLING OF TREATED TIMBER. THE POINT OF PIKES, PEAVES ETC. MUST NOT BE DRIVEN INTO THE SURFACES OF TREATED TIMBER.

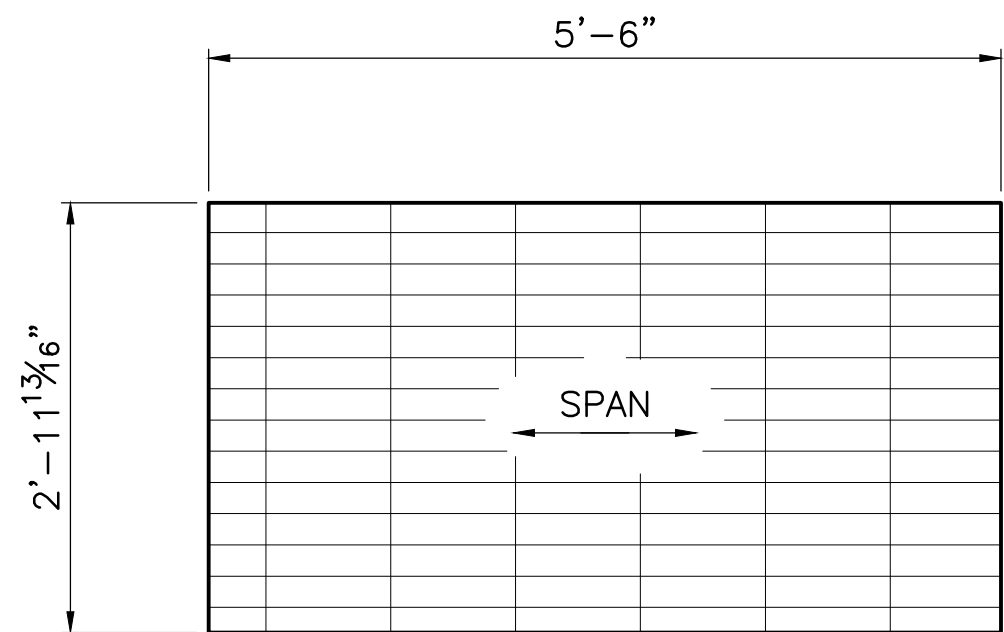
- FOR NOTES AND SPECIFICATIONS SEE DRAWING No.-1.9
- “S1S” DENOTES SIZED ONE SIDE
- ALL TIES SHALL BE CUT TO EXACT LENGTHS AS SHOWN;
- ALL TIES SHALL COME WITH ANTI-SPLIT PLATES INSTALLED AT BOTH ENDS;
- MARKS AND ORIENTATION SHALL BE CLEARLY INDICATED ON ALL PIECES;

—BRIDGE TIE 10x14x14'-0" (A5)	49 EA
—REFUGE BAY TIE 10x14x18'-10" (B5)	8 EA
—BRIDGE TIE 10x14x14'-0" (C5)	91 EA
—BRIDGE TIE 10x14x13'-0" (D5)	86 EA
—BRIDGE TIE 10x14x13'-0" (E5)	176 EA
—REFUGE BAY TIE 10x14x18'-10" (F5)	4 EA
—BRIDGE TIE 12x14x13'-0" (G5)	4 EA
—BACKWALL TIE 8x14x8'-6" (H5)	2 EA

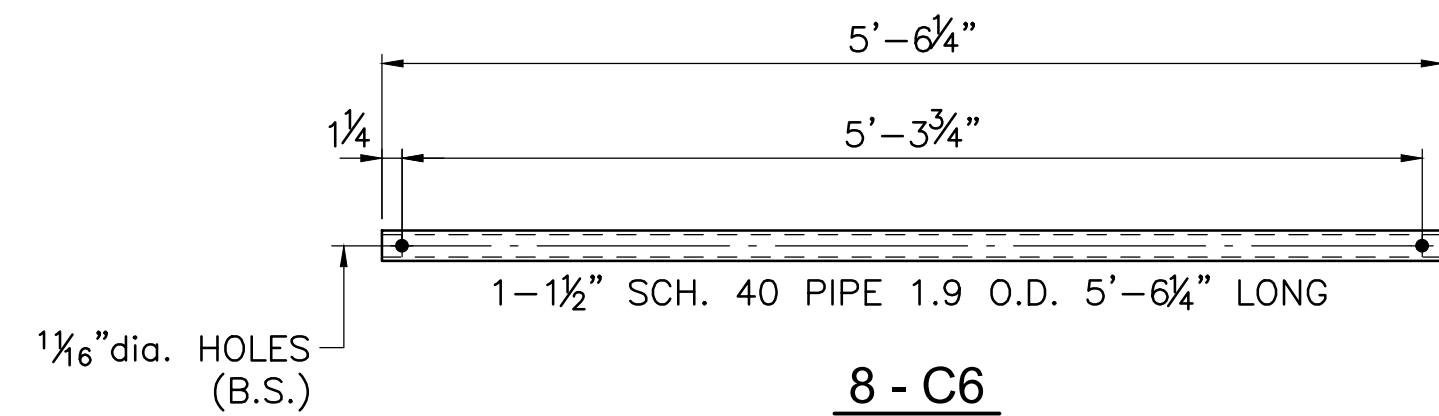
[illegible]



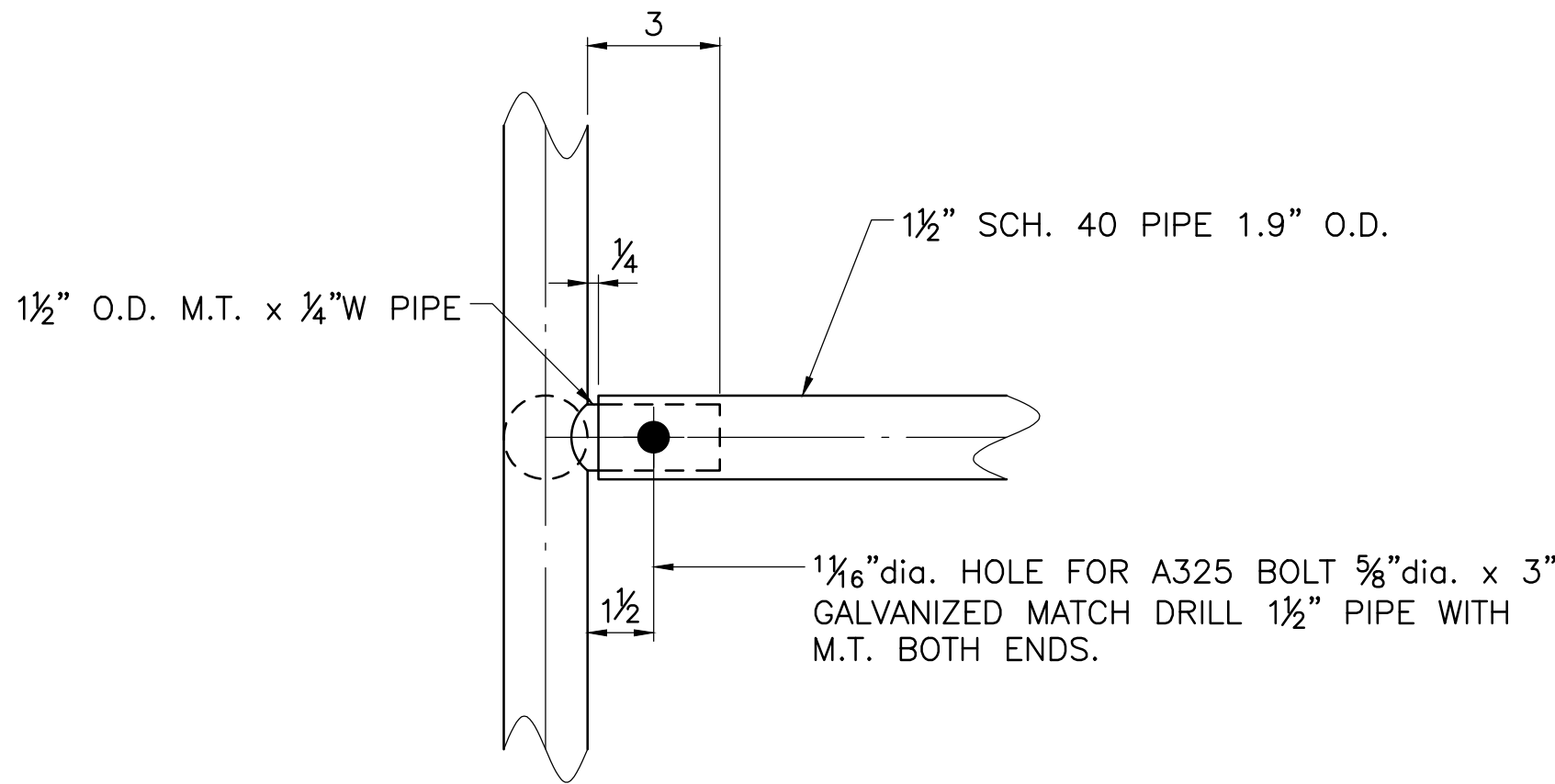
4 - A6
SCALE: 3/4"=1'-0"



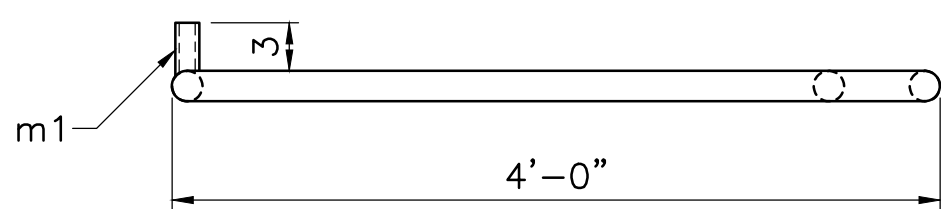
4 - B6
SCALE: 3/4"=1'-0"



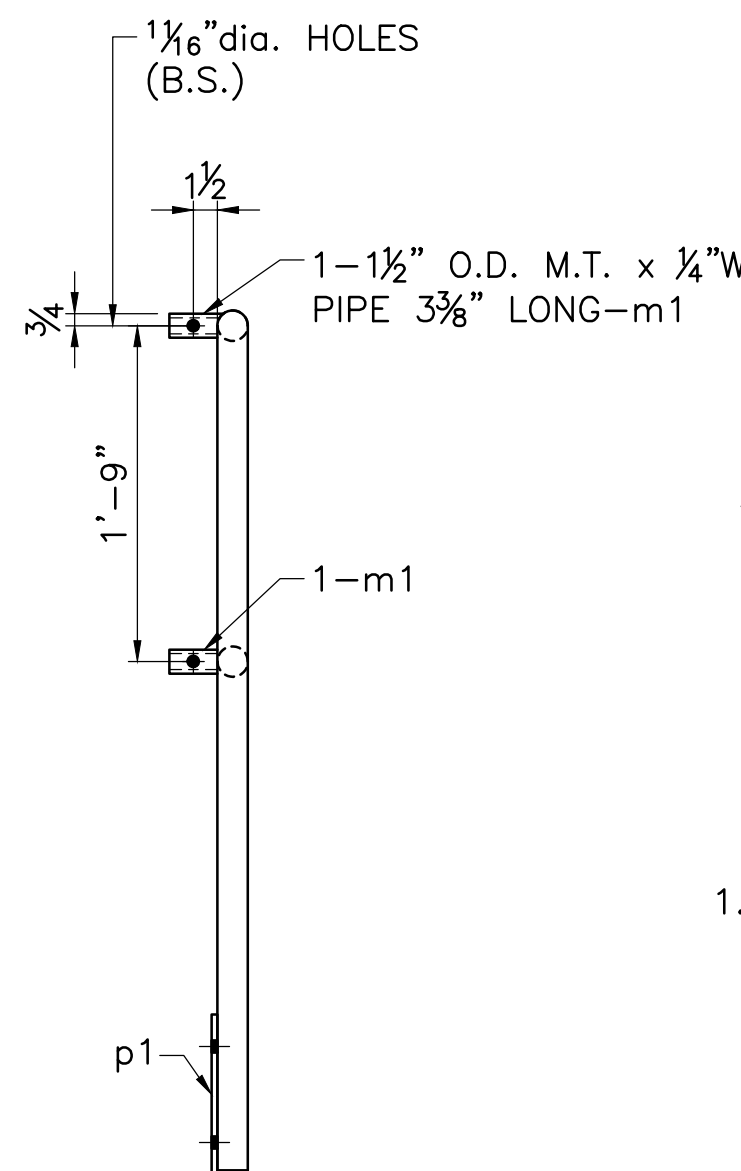
8 - C6
UNIT WT: 15 LBS



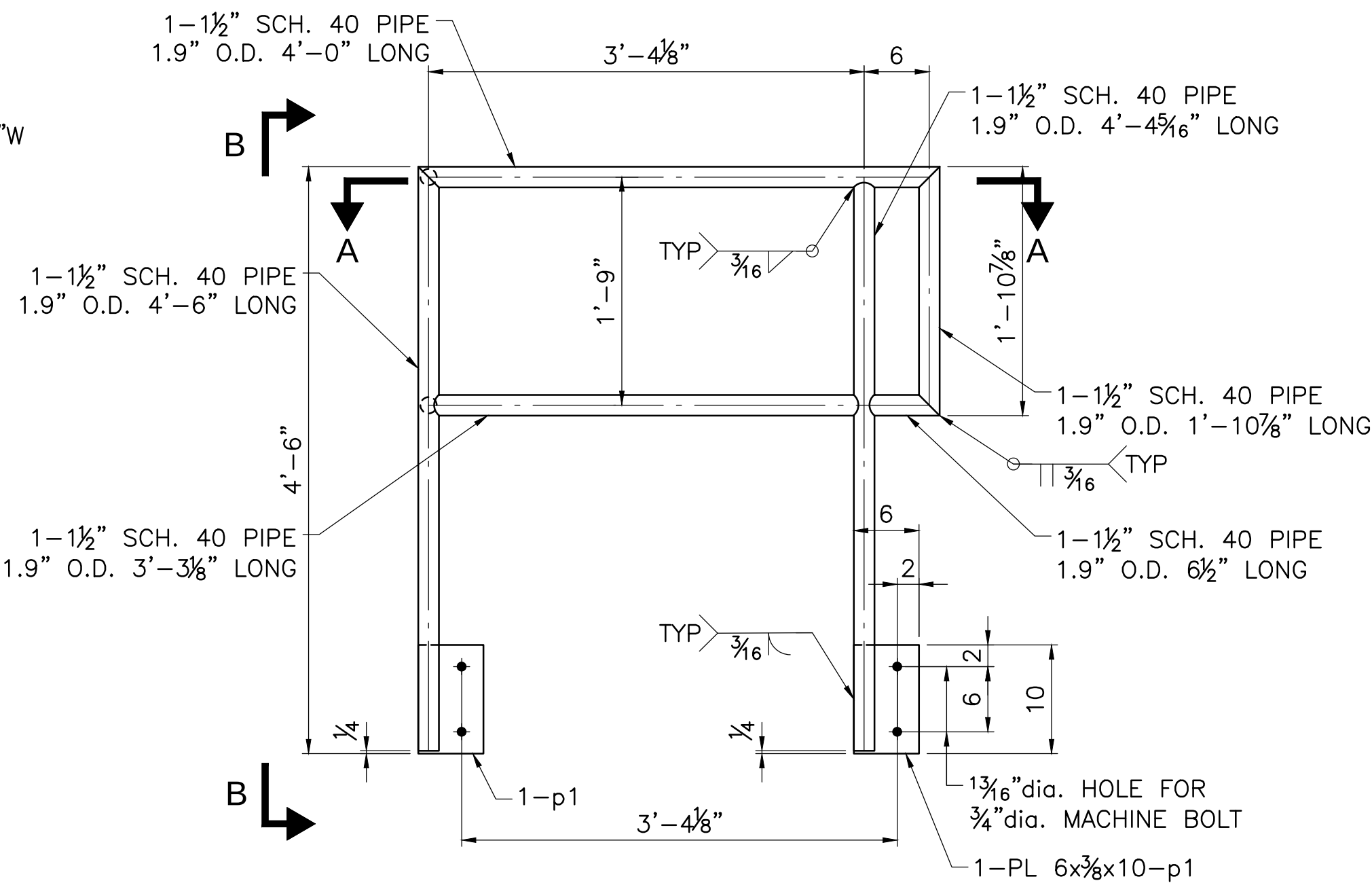
HANDRAIL CONNECTION DETAIL
SCALE: 3"=1'-0"



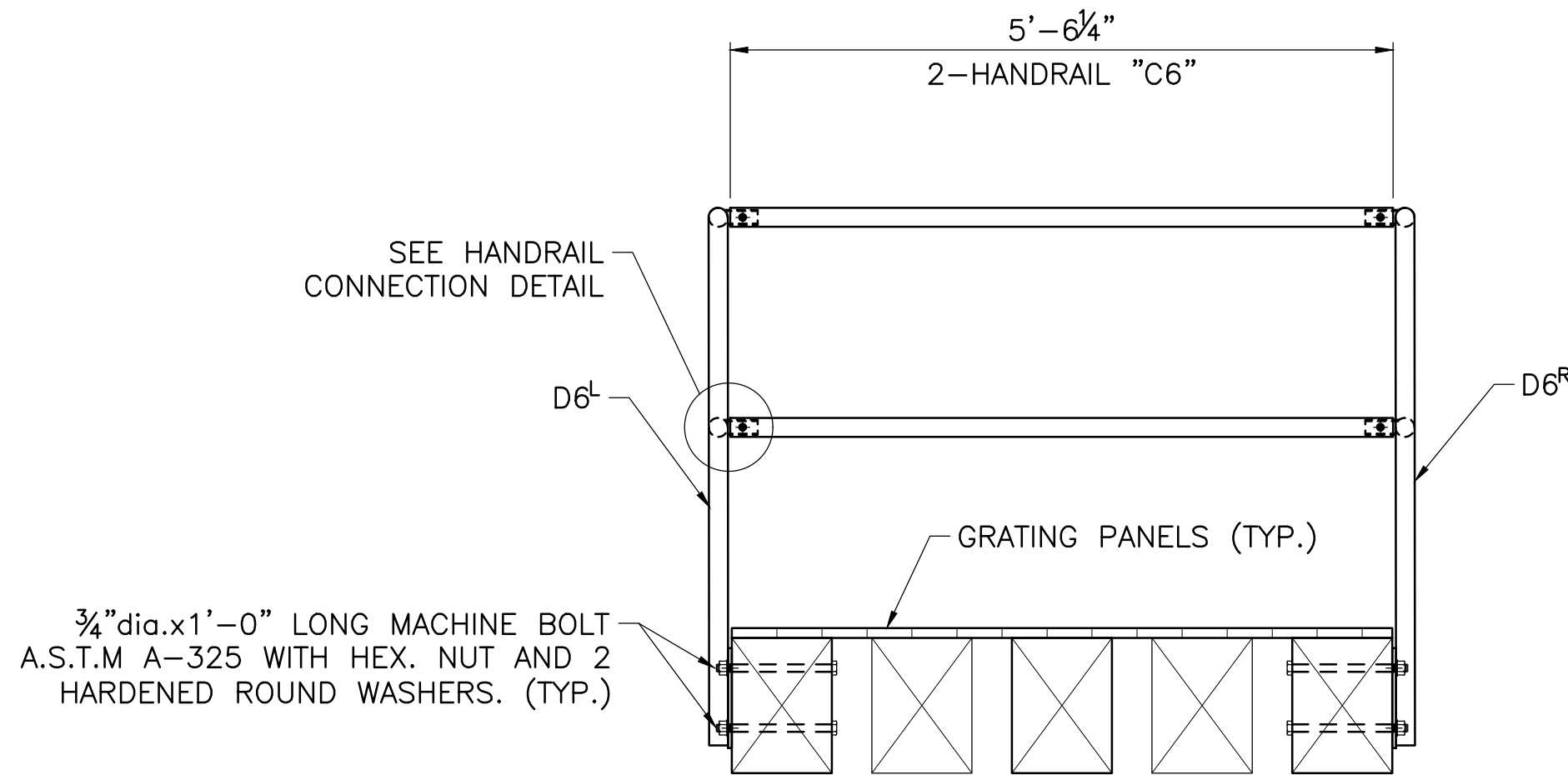
VIEW "A-A"



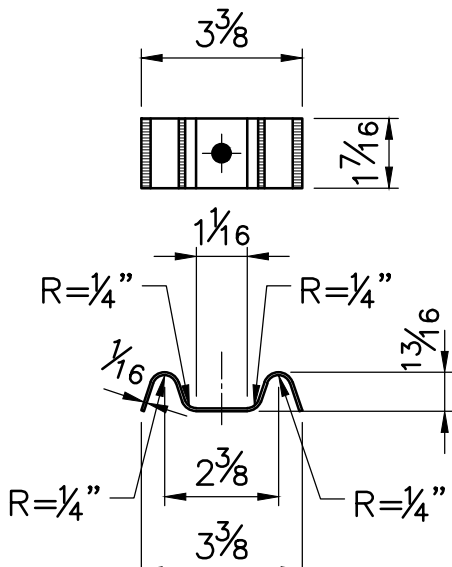
VIEW "B-B"



4 - D6^R RIGHT (AS SHOWN)
4 - D6^L LEFT (FROM SHOWN)
UNIT WT: 65 LBS



TYPICAL REFUGE BAY ASSEMBLY DETAIL
SCALE: 3/4"=1'-0"



AMICO-KLEMP STANDARD TYPE H-3 SADDLE CLIP
c/w 1/4" dia. x 6" LAG SCREW
(GALVANIZED)

65 - E6

SCALE: 3"=1'-0"

SPECIFICATIONS:

- GRATING SHALL BE TYPE 19-4 FROM AMICO-KLEMP WITH 1x3/8" BEARING BARS WELDED AT 1 3/8" C/C AND CROSS BARS AT 4" C/C. GRATING SHALL BE FULLY Banded AND HOT-DIP GALVANIZED. GRATING PANELS SHALL BE SECURED USING 1/4" dia. x 6" LAG-SCREWS AND "E6" GRATING CLIPS PER REFUGE BAY TIE. (APPROX. WT.= 7.23 LBS PER FT²)
- HANDRAIL TO BE 1 1/2" SCHEDULE 40 PIPE 1.9" OUTSIDE DIAMETER (UNLESS NOTED OTHERWISE);

ESTIMATED QUANTITIES:(ON THIS DRAWING)

- STRUCTURAL STEEL _____ 640 LBS
- GRATING _____ 161 FT²
- 3/4" dia. x 1'-0" LONG MACHINE BOLT A.S.T.M A-325 WITH HEX. NUT AND 2 HARDENED ROUND WASHERS _____ 40 EA.
- 5/8" dia. x 3" A325 BOLT (FOR "C6") _____ 20 EA.

NOTES:

- FOR NOTES, SPECIFICATIONS AND INSTALLATION OF REFUGE BAYS SEE STANDARD ONR DRAWING: A-6264;
- ALL MATERIAL ON THIS DRAWING SHALL BE HOT-DIP GALVANIZED;
- FOR OTHER NOTES AND SPECIFICATIONS, SEE DRAWING No.-2.1;

NOTES:

LEGEND:

O.N. Rail

HATCH

PROJECT No. H-351632

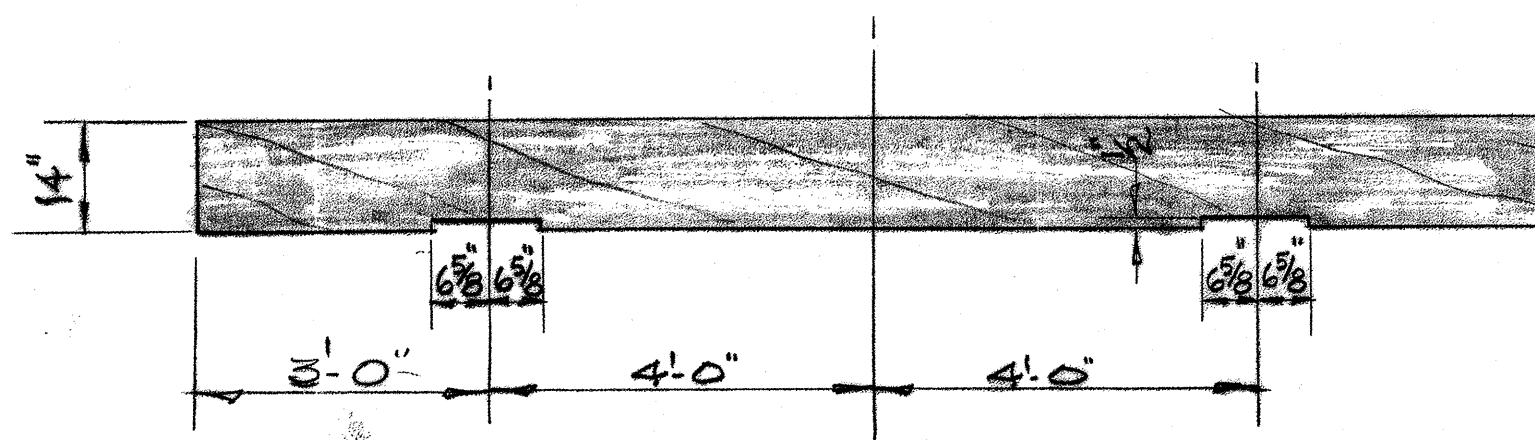
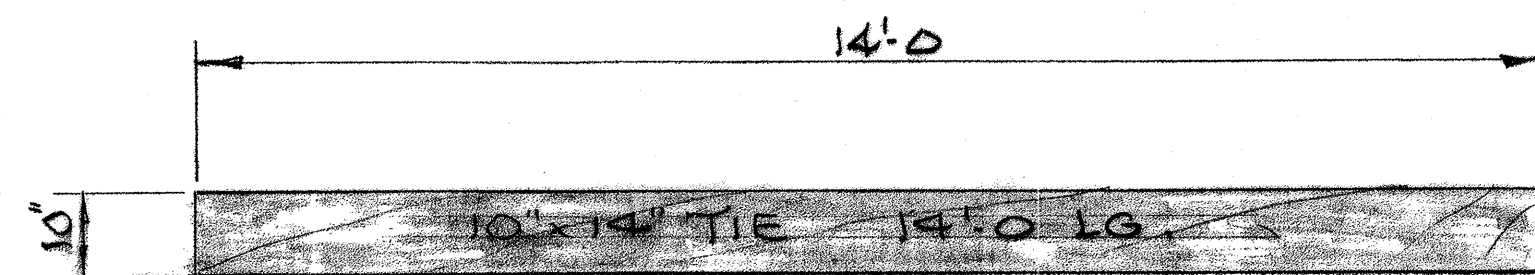
BRIDGE OVER DRIFTWOOD RIVER
Mile 79.00 RAMORE SUBDIVISION

DECK REPLACEMENT
FIELD AND STEEL DETAILS

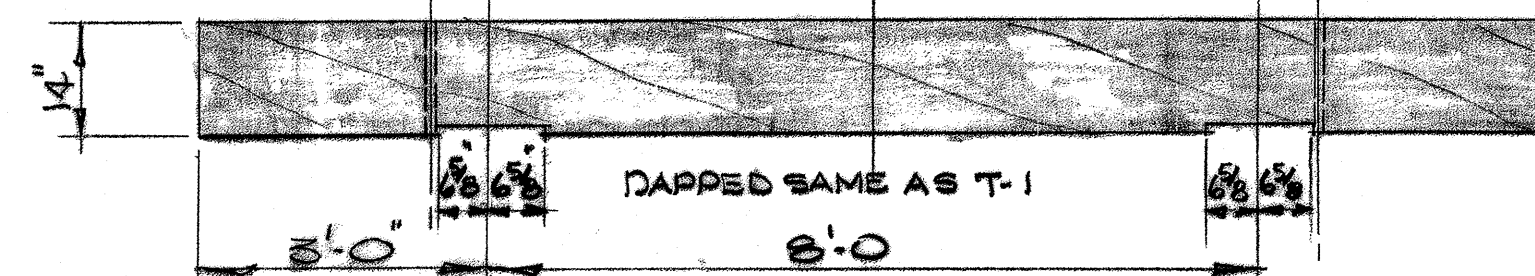
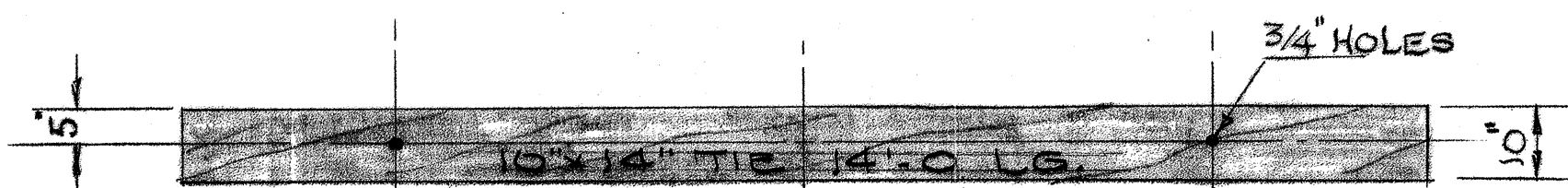
DWG. BY: ED	CHK BY: WW	OFFICE FILE:
		SCALE: 1"=1'-0" U.N.
ENGINEER:		DATE: MAY 2016
		PLAN No.
PROJECT MANAGER:		RMR-79.00-2.6

No.	DATE	REVISION	BY
	2018/06/18	AS-BUILT	AM
1	2016/09/22	ISSUED FOR CONSTRUCTION	LL
0	2016/05/19	ISSUED FOR TENDER	ED

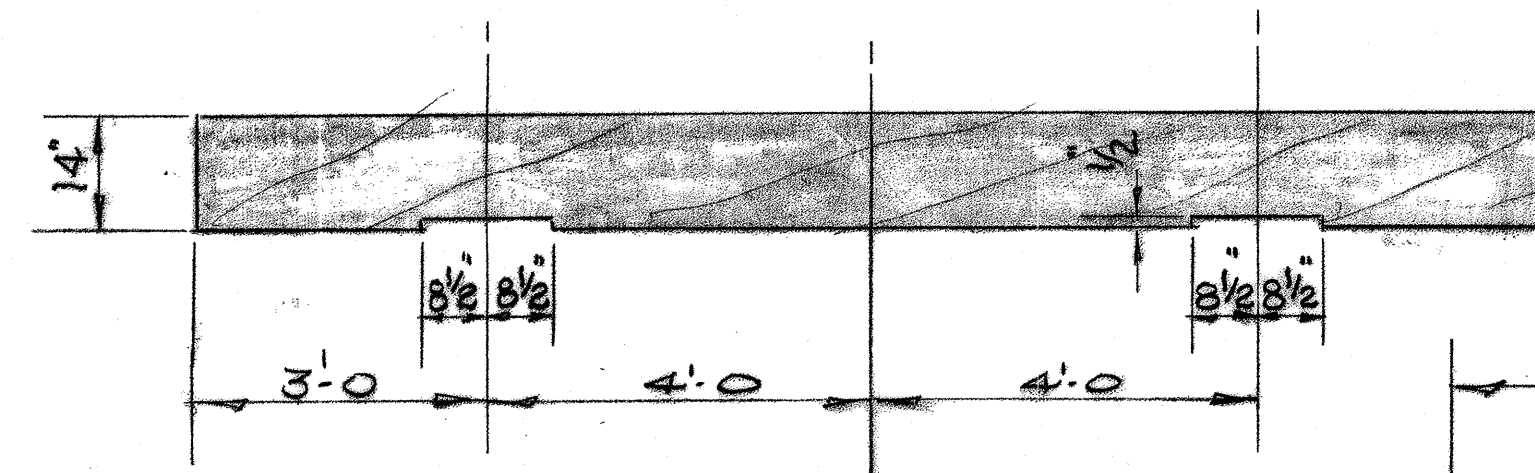
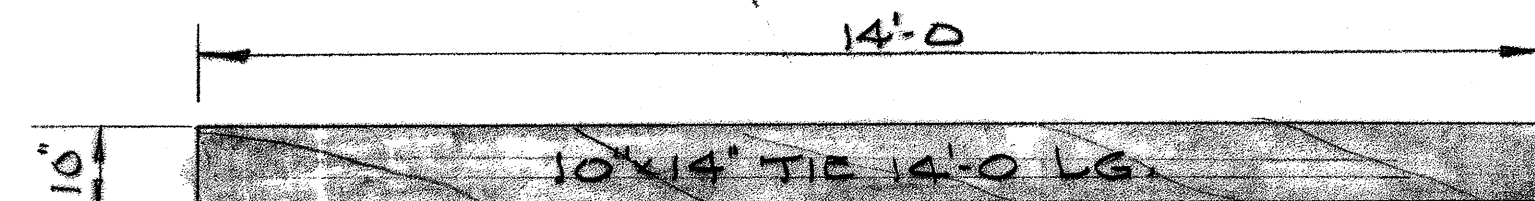
B-1308



T-1 114 REQ'D.



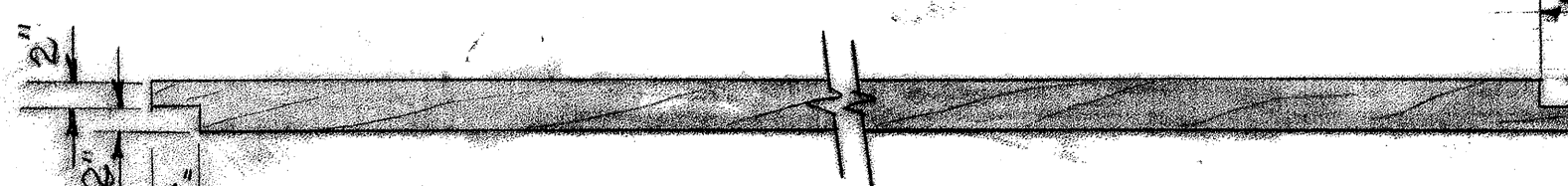
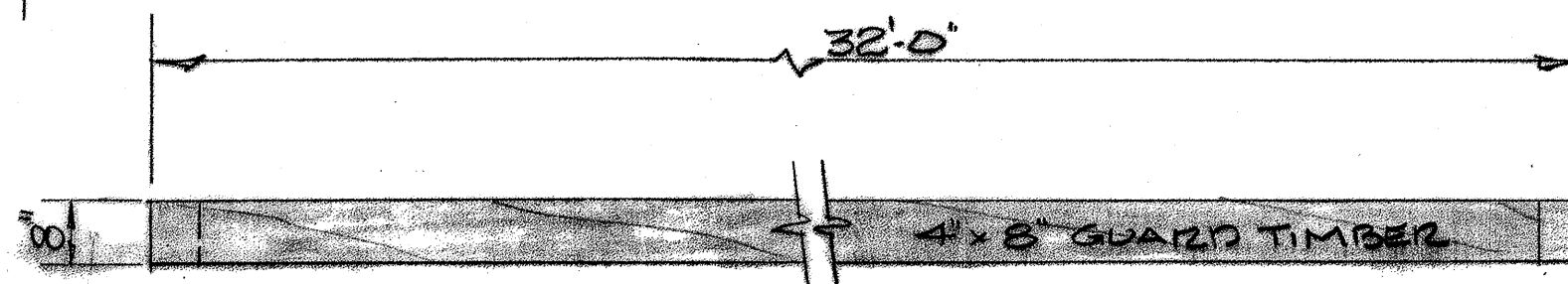
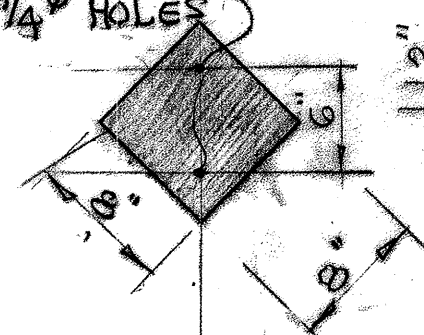
T-2 40 REQ'D.



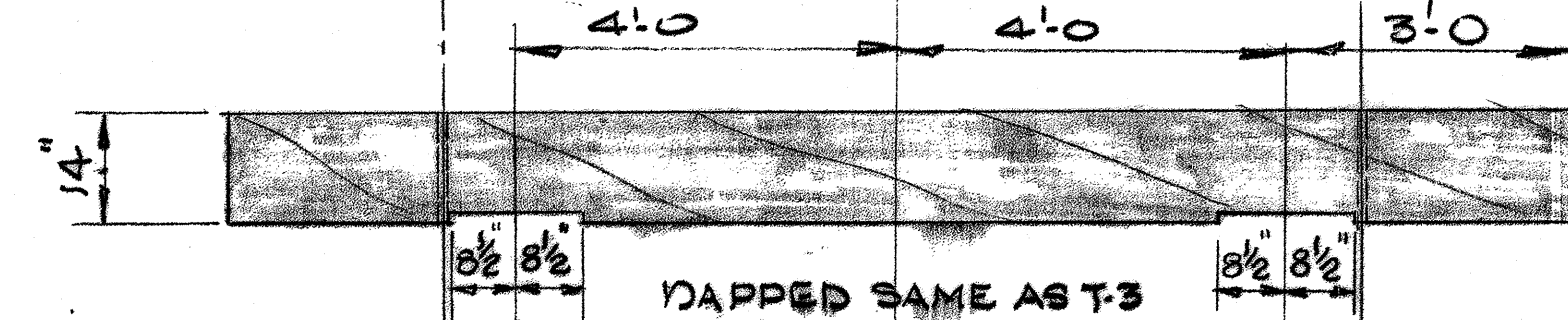
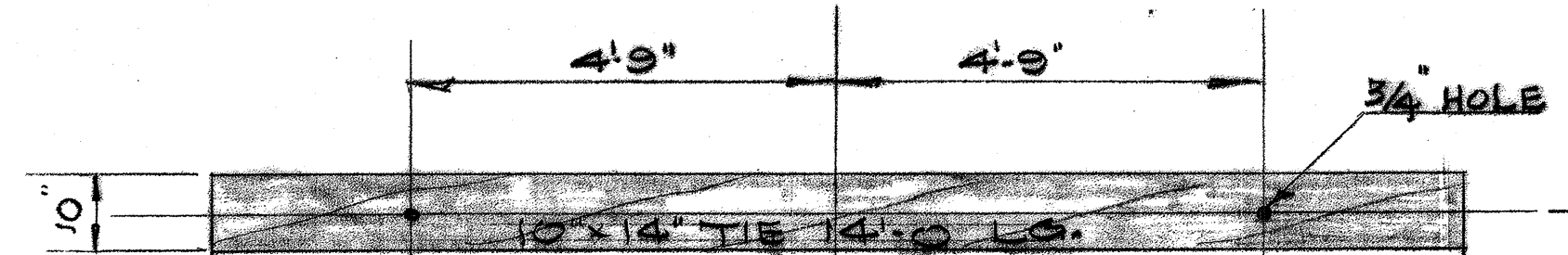
T-3 57 REQ'D.

SPACER BLOCKS
4"x8"-8"lg. fir
pressure treated
475 REQ'D.

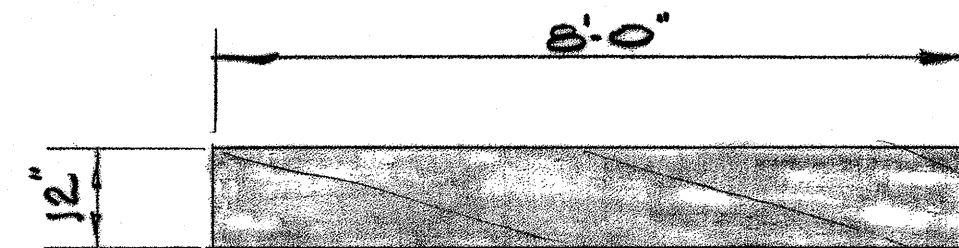
DRILL 1/4" HOLES



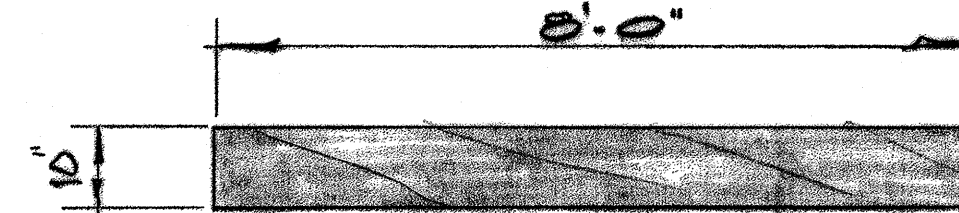
4x8 GUARD TIMBER 18 REQ'D.



T-4 20 REQ'D.



T-5 1 REQ'D.

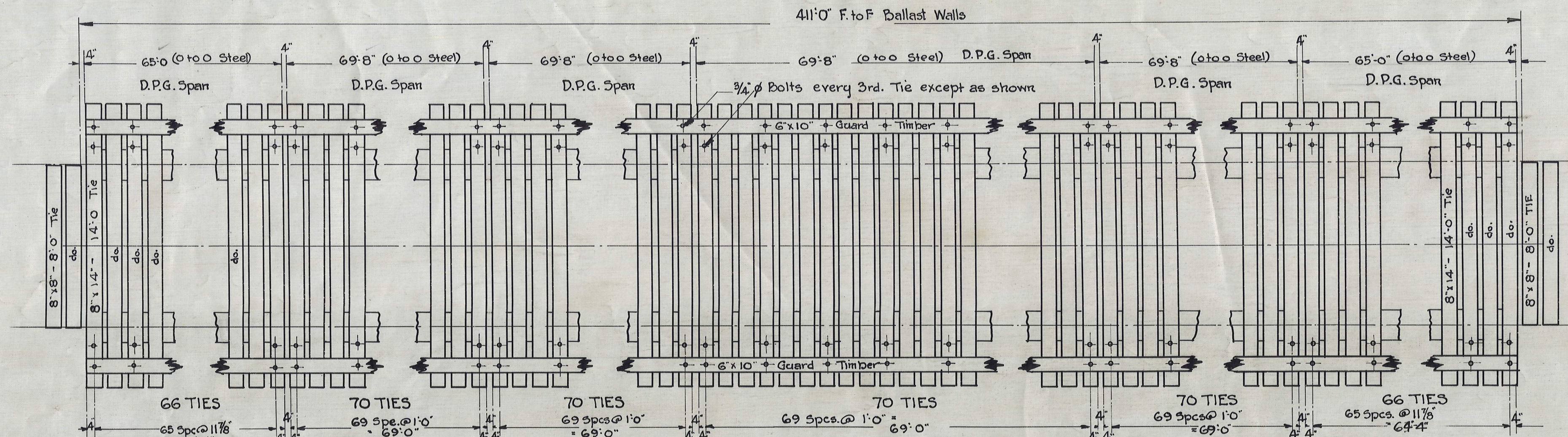


T-6 1 REQ'D.

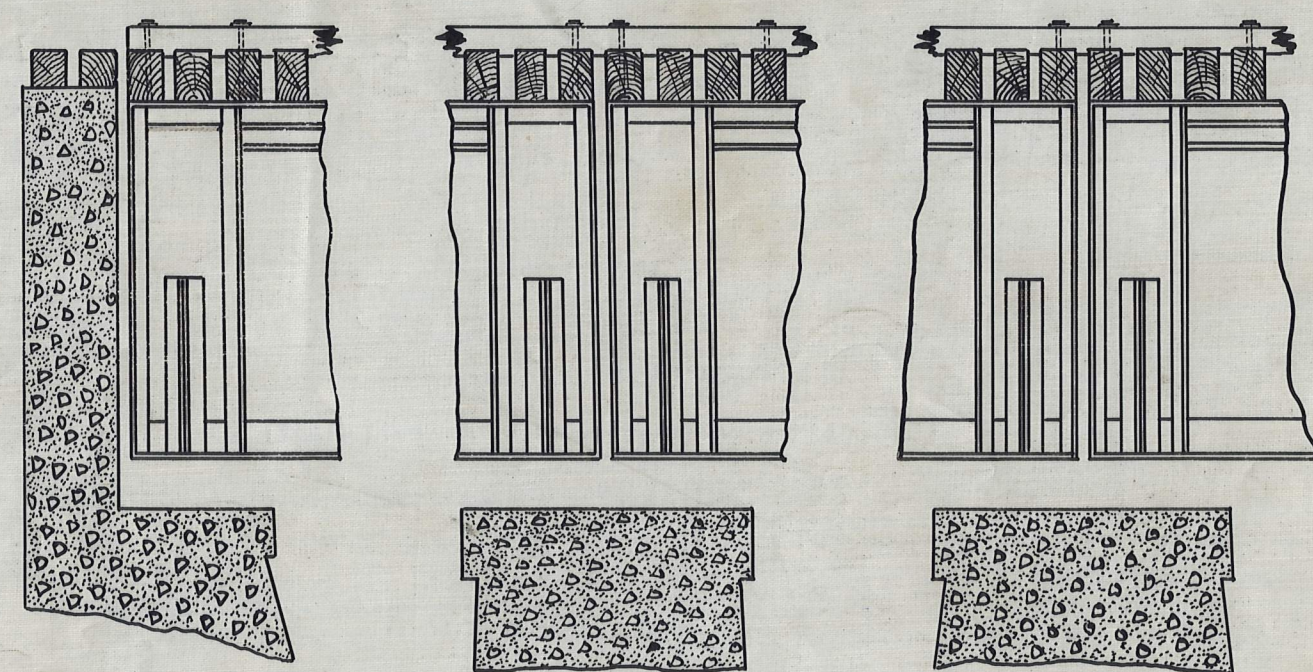
NOTE:
ALL DRILLING DAPPING AND
CUTTING SHALL BE COMPLETED
PRIOR TO PRESSURE TREATMENT.

ONTARIO NORTHLAND R.L.WY.		
ENGINEERING DEPT. NORTH BAY ONT.		
DECK TIMBER DETAILS		
SOUTH WABSI BRIDGE MP 115.47		
TEMAGAMI SUBDIVISION		
drn WDA	chk	
approved		Struct. Supt.
Scale as noted		date June 7/78
		no 131308

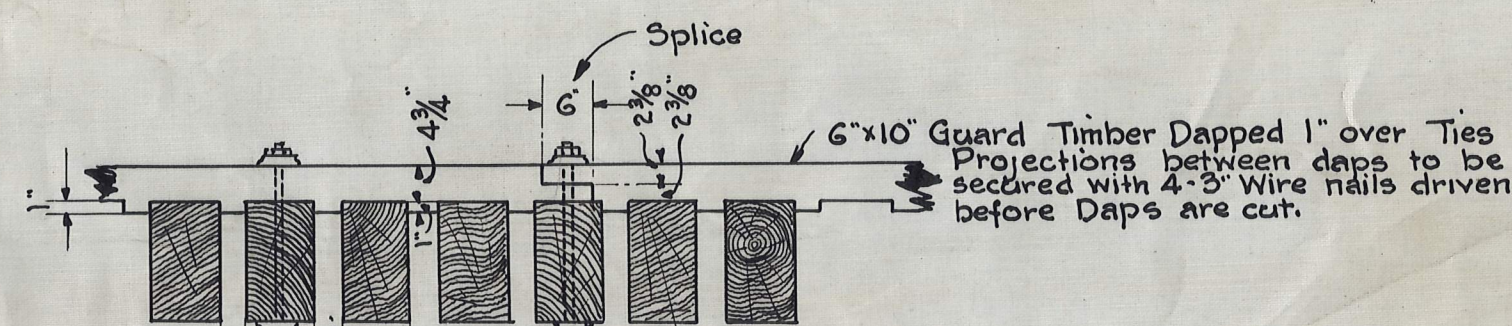
C-218



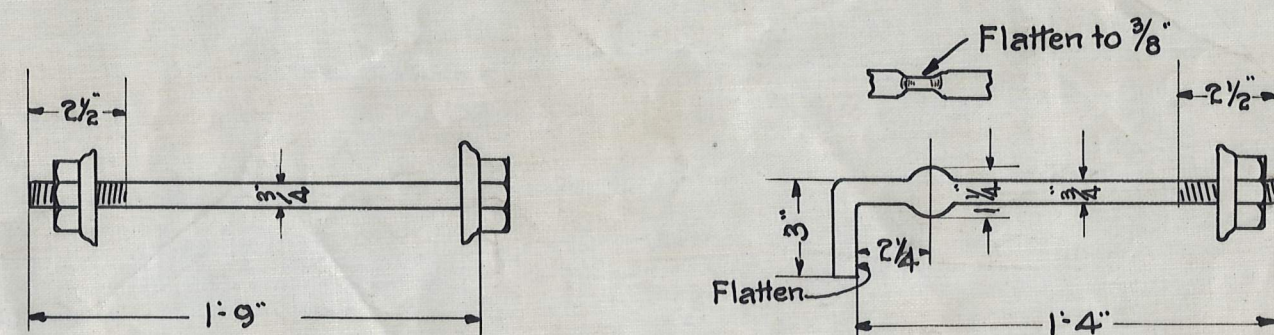
PLAN OF DECK



SECTION

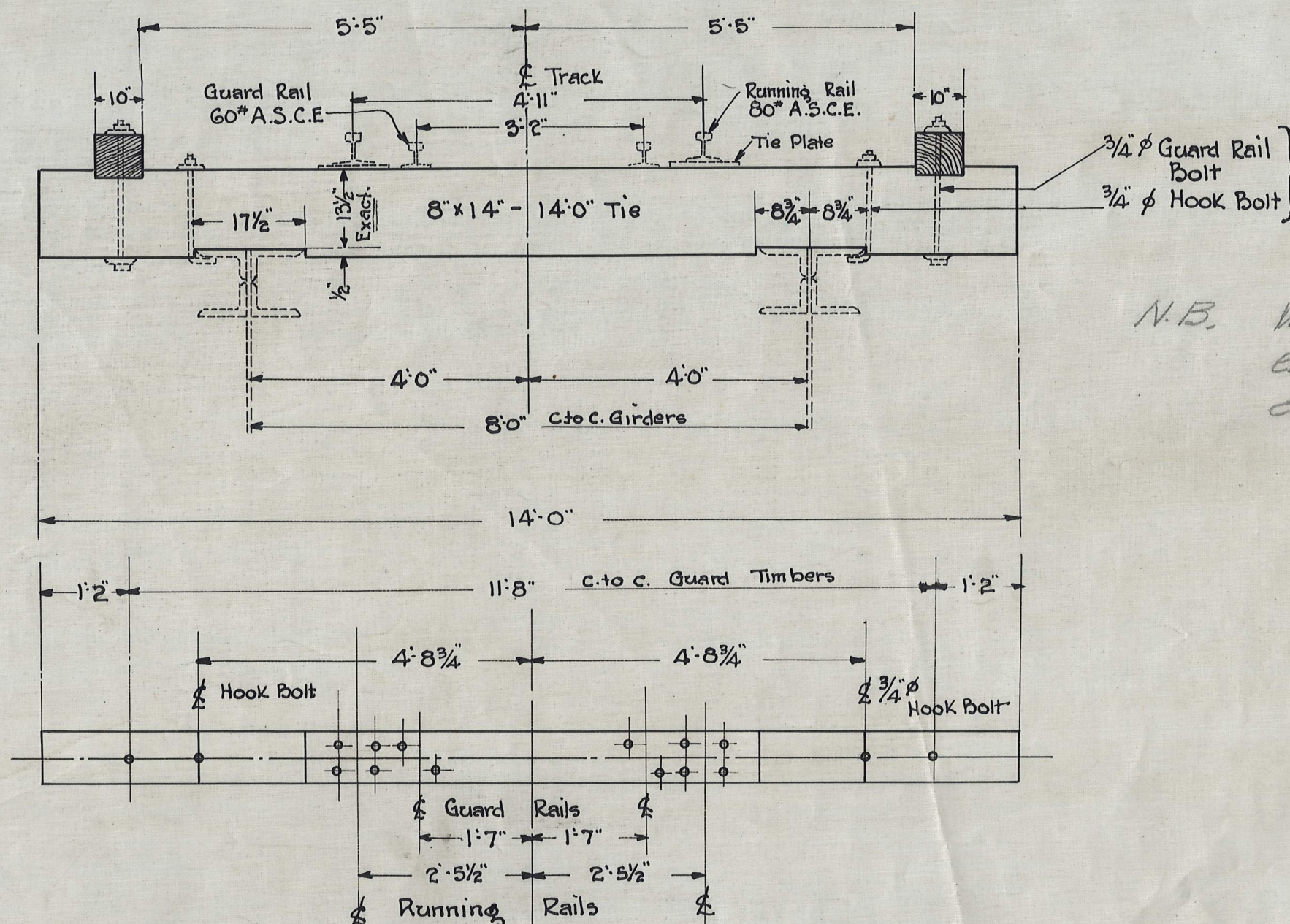


DETAIL OF TIMBER GUARD



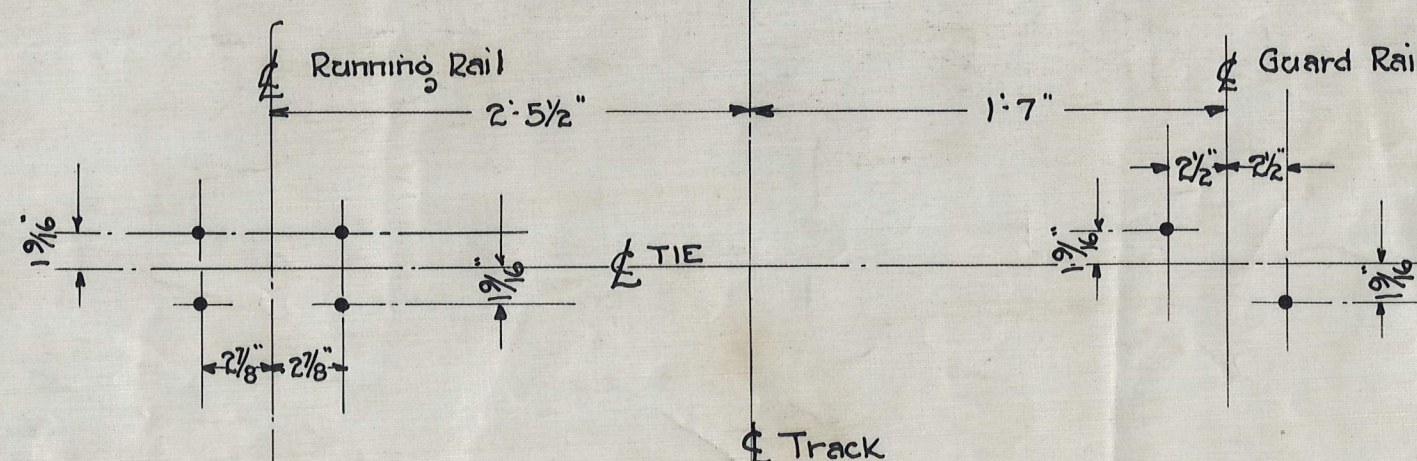
284 - 3/4" ϕ x 1'-9" bolts
568 - C.I. Washers
284 - Hex. Nuts

284 - 3/4" ϕ Hook Bolts
284 - Hex. Nuts
284 - C.I. Washers



These bolts to be generally every 3rd. Tie except as shown on plan.

N.B. When dapping guard timbers note that on the two end 65' spans ties are spaced @ 11 7/8" c/c and on remaining spans @ 12" c/c.



TEMPLET for boring for 5/8" ϕ Track Spikes

DETAIL OF 8" x 14" x 14'0" TIE

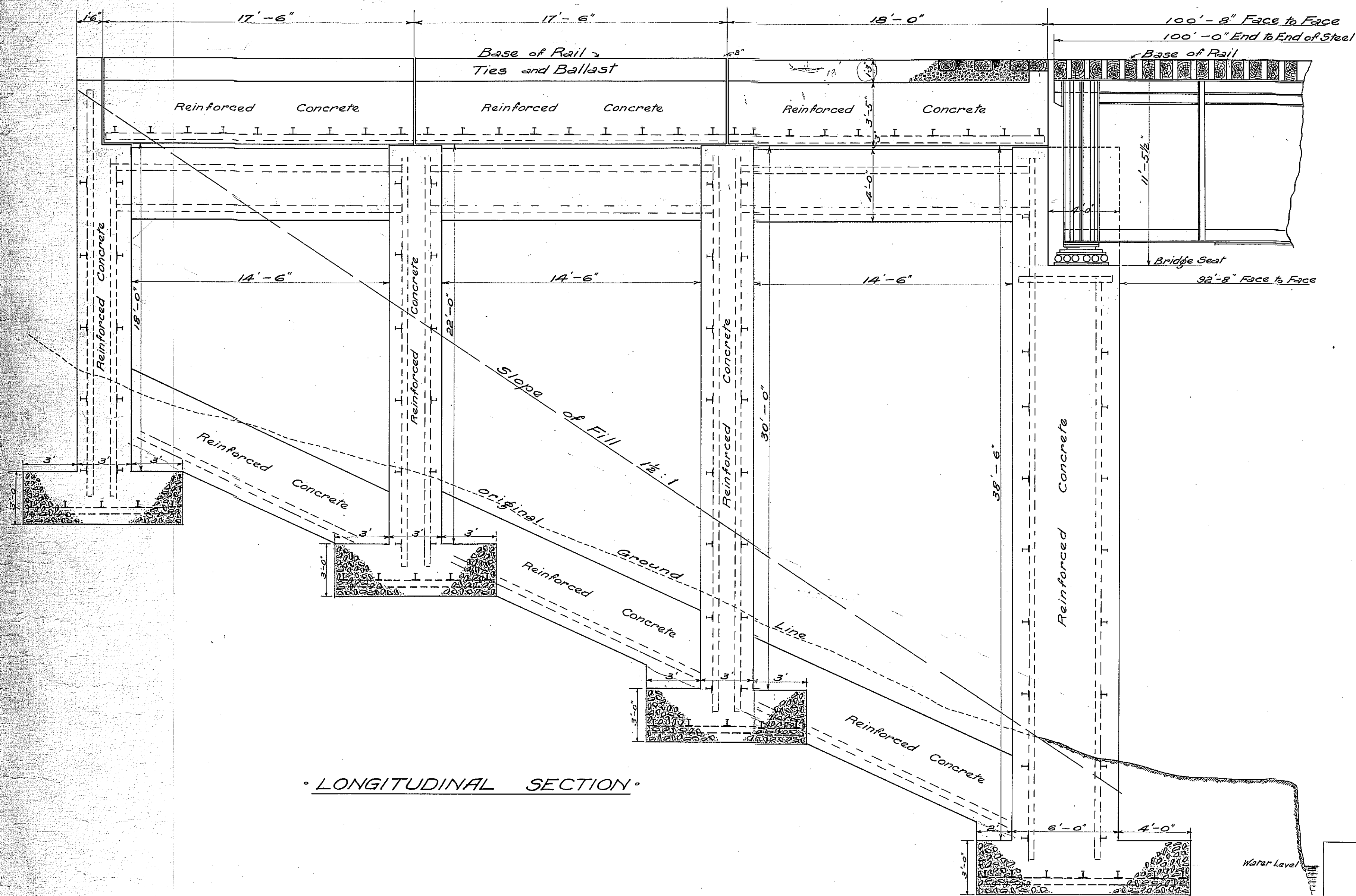
BILL OF MATERIAL
IN DECK

412 - 8" x 14" x 14'0" Ties
4 - 8" x 8" x 8'0" "
2 - 6" x 10" x 4'0" Guard Timbers (Spliced)
284 - 3/4" ϕ Bolts for Guard Timber.
284 - 3/4" ϕ Hook Bolts
852 - C.I. Washers
568 - Hex. Nuts.

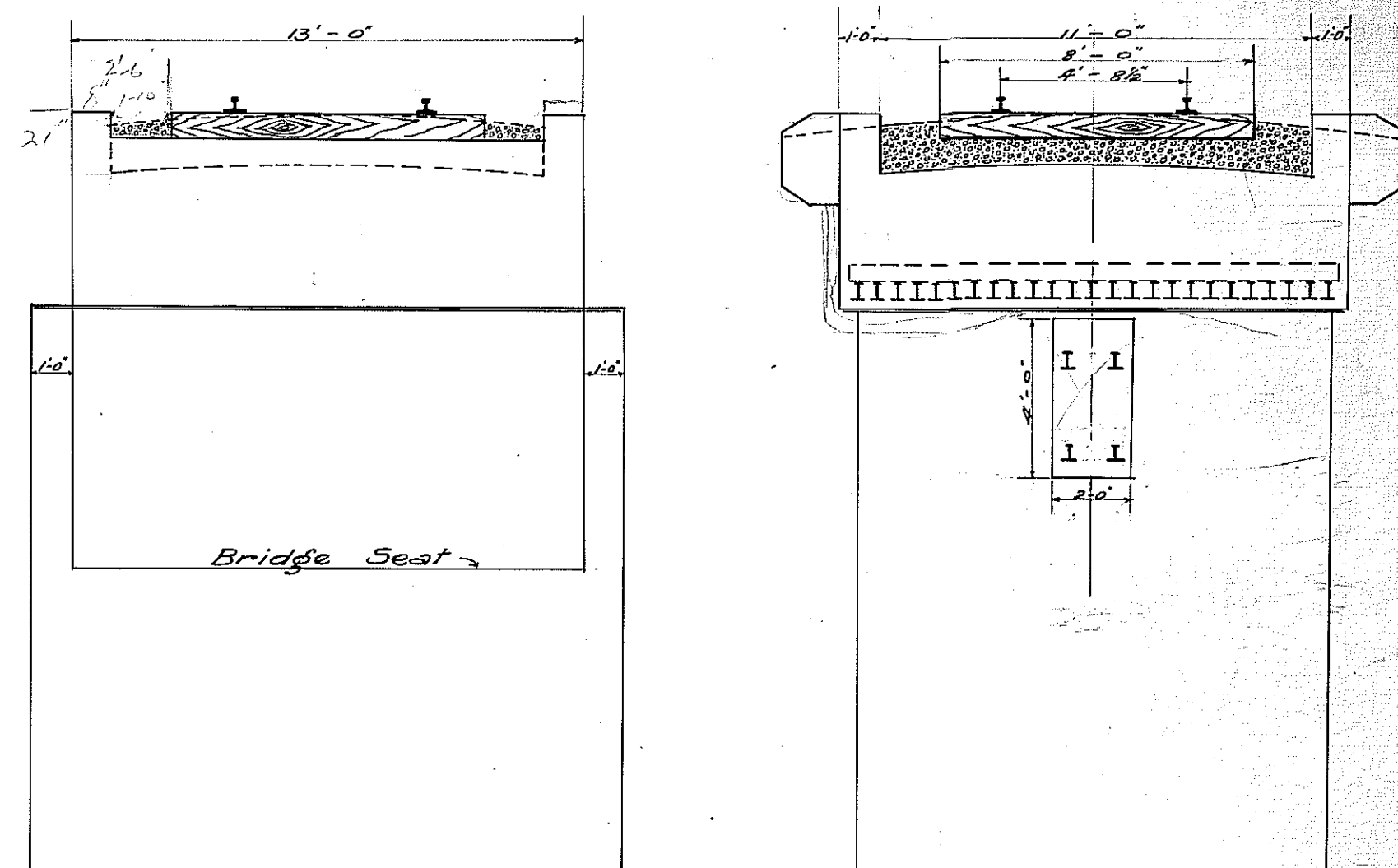
TEMISKAMING & NORTHERN ONTARIO RLY.
BRIDGE OVER THE FREDERICK
HOUSE RIVER
MILE 10 - PORCUPINE BRANCH
DETAIL OF DECK

OFFICE OF CHIEF ENGINEER, NORTH BAY, ONT.
Appr: *[Signature]*

Cut along this line

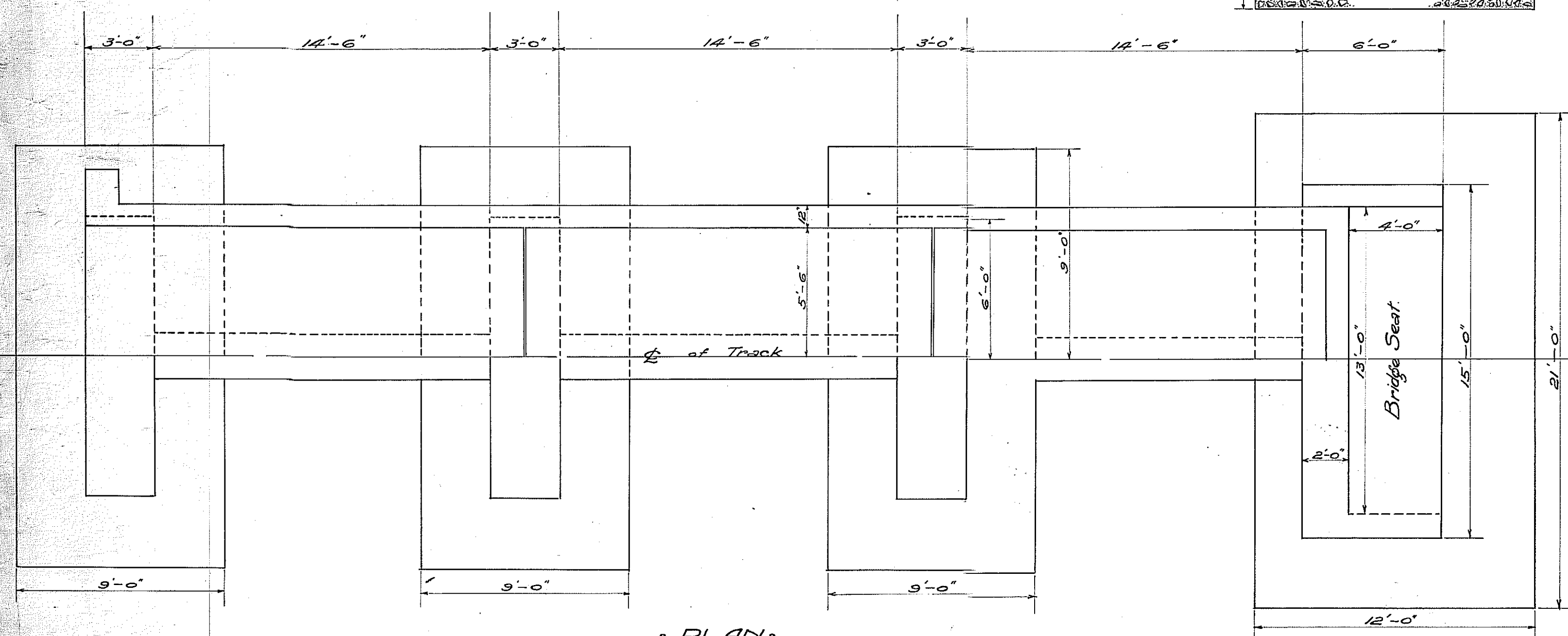


• LONGITUDINAL SECTION •



• CROSS SECTION •

• ELEVATION •



• PLAN •

T.&N.O.R.
PLAN
OF
TRESTLE ABUTMENT
FOR
PORCUPINE RIVER CROSSING
MILEAGE 17.7
PORCUPINE BRANCH
Scale: 1/4 inch to one foot.

Chief Engineer's Office,
North Bay: April 18th 1912.

CHIEF ENGINEER.

E-4-5

R-239
RYA